

LIBERTY THEATRE – PHASE SEVEN YOUTH THEATRE / PROGRAM CENTER – ADDITION

2100 SHERMAN AVE.

NORTH BEND, OREGON 97459

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JOB NO. 1529.7

SET NO.

PROJECT DESCRIPTION

JURISDICTION: CITY OF NORTH BEND, OREGON (ENGINEERING DEPARTMENT)
835 CALIFORNIA ST., NORTH BEND, OREGON 97459
(541) 756-8505

UTILITY PROVIDER(S): NORTHWEST NATURAL GAS, PACIFIC POWER,
COOS BAY-NORTH BEND WATER BOARD

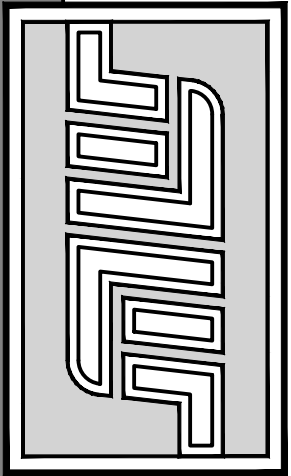
BUILDING AREAS: LOWER STORY - 3,722 SQ. FT. (TABLE 503)
UPPER STORY - 3,401 SQ. FT.
TOTAL FIRE AREA - 7,642 SQ. FT. (SEC. B104, 2022 OFC)
ENCL. BLDG. AREA - 1,510 SF (PH. 2); 2,212 SF (PH. 4) (SEC. 502)
NEW UPPER FLOOR - 3,401 SF (PH. 7)

DESIGN CRITERIA		2022 OSSC	
TYPE OF CONSTRUCTION	TYPE 5-A NON-FIRE-SPRK		
USE & OCCUPANCY	A-3		
SEPARATION (HOURS)	2 HR F.-R. RATING		
HEIGHT LIMITATIONS	2-STORY, 50 FT. ACTUAL: 1-STORY, 27.5 FT.		
SEISMIC BASE SHEAR	CATEGORY / SITE CLASS D / Sds = 1.65 / Sd1 = 1.16 TYPE: V=0.286W (ASCE 7-16, T-12.14.8.1)		
WIND PRESSURE	WIND SPEED = 120 MPH, EXP. B, I = 1.0, P = 23.00 MPH		
LOAD TRANSFER	ROOF	DEAD	15 PSF
		LIVE	20 PSF (SNOW)
		COLLATERAL	5 PSF
FLOOR	DEAD	17 PSF (EXISTING)	
	LIVE	100 PSF (EXISTING)	
SNOW DRIFT	FLAT ROOF (PF = 14.0) 1/4' PER FOOT		
SOIL BEARING	CLASSIFICATION: SM-SC / BEARING: 2,000 PSF		
WINTER DESIGN TEMP.	20 DEG.		
SUMMER DESIGN TEMP.	90 DEG.		
DEGREE DAYS	4,856 HEATING + 65 DEG. BALANCE TEMPERATURE		
ELEVATION	GROUND FLOOR FINISH ELEVATION = 55.00'		
FIRE CODE	2022, OREGON FIRE CODE		
PLUMBING CODE	2021, OREGON PLUMBING SPECIALTY CODE		
MECHANICAL CODE	2022, OREGON MECHANICAL SPECIALTY CODE		
ELECTRICAL CODE	2021, OREGON ELECTRICAL SPECIALTY CODE		
ENERGY CODE	2021 OREGON ENERGY EFFICIENCY SPECIALTY CODE		

OWNER:

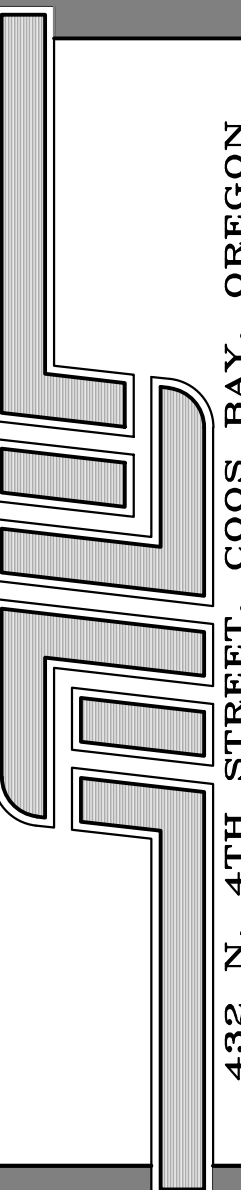
LITTLE THEATER ON THE BAY, INC.
P.O. BOX 404
NORTH BEND, OREGON 97459
PHONE: (541) 756-4336

CONTACT: JEANNE WOODS, RESTORATION COMMITTEE CHAIR
(541) 297-3525



PROJECT TITLE:
YOUTH THEATRE / PROGRAM CENTER
LIBERTY THEATRE - PH. 7 ADDITION
2100 SHERMAN AVE.
NORTH BEND, OREGON 97459

McSWAIN - WOODS ARCHITECTURE



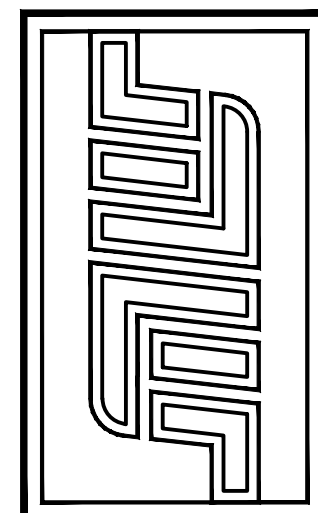
432 N. 4TH STREET, COOS BAY, OREGON
(541) 269-0618 (M) 503.708.3440 Email: stu@mcswn-woods.com

SHEET TITLE:
PROJECT DESCRIPTION
DRAWING INDEX
VICINITY MAP

DATES:	FEB. 20, 2023
PLAN REVIEW	6/25/23
ISSUE/BID	5/10/23
CONST. SET	/
REVISION A	8/30/23
REVISION B	/
REVISION C	/

PROJECT #
1529.7

G1.1



ARCHITECT:

McSWAIN-WOODS ARCHITECTURE

432 N. 4TH STREET
COOS BAY, OREGON 97420

(Ph) 541.269-0618 (M) 503.708-3440

stu@mcswn-woods.com

CONSTRUCTION MANAGER:

SCOTT PARTNEY CONSTRUCTION

720 CHAPPELL PARKWAY
NORTH BEND, OREGON 97420
(541) 756-7060

STRUCTURAL ENGINEER

STUART A. WOODS, AIA
432 N. 4TH STREET
COOS BAY, OREGON 97420
(541) 269-0618

PLUMBING CONTRACTOR

DESIGN BUILD SUBCONTRACTOR

HVAC CONTRACTOR

DESIGN/BUILD SUBCONTRACTOR

ELECTRICAL CONTRACTOR

DESIGN/BUILD SUBCONTRACTOR

PHASE SEVEN ADDITION

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CODE DATA:	
BUILDING CODE:	2022 OREGON STRUCTURAL SPECIALTY CODE ASCE STANDARD 7-16 (DESIGN LOADS)
MECHANICAL CODE:	2022 OREGON MECHANICAL SPECIALTY CODE
PLUMBING CODE:	2021 OREGON PLUMBING SPECIALTY CODE
ELECTRICAL CODE:	2021 OREGON ELECTRICAL SPECIALTY CODE
FIRE CODE:	2022 OREGON FIRE CODE

CONSTRUCTION DATA:	
TYPE OF CONSTRUCTION:	TYPE V-A (NON-F.S.) O88C SEC. 602
BUILDING HEIGHT:	21'-6"
ALLOWABLE BUILDING HEIGHT:	50'-0" O88C TABLE 504.3 SEC. 504.4
BUILDING STORIES:	1 STORY
ALLOWABLE (MAX.) BUILDING STORIES:	2-STORY O88C TABLE 504.4
SETBACKS:	0' - 3.8' - 5.2' - 53.7' O88C SEC. 506 TABLE 602

BUILDING AREA:	
1. WITHIN EXTERIOR WALLS: (2ND STORY)	3,920 SQ. FT.
2. BY DEFINITION:	11,500 SQ. FT. - PER FLOOR O88C SEC. 506 TABLE 506.2
3. ALLOWABLE AREA:	3,920 = 0.341 < 1 O88C SEC. 506.3
	11,500

FIRE RESISTIVE ELEMENTS:	
EXTERIOR WALLS	F. R. RATING = 1 HR. EXCEPT EAST WALL N. R. (+ 30 FT.) O88C TABLE 601 TABLE 1005.5
OCCUPANCY SEPARATION	F. R. RATING = N. A. O88C TABLE 508.4
FIRE WALLS & BARRIERS	F. R. RATING = 2 HR. O88C TABLE 106.4

USE & OCCUPANCY CLASSIFICATION:	
OCCUPANCY:	A-3 ANCILLARY TO THEATER AREA = 3,122 SF (LOWER FLOOR) 3,920 SF (UPPER FLOOR) O88C SEC. 302
OCCUPANT LOAD:	SEE ROOM AREA TABLE, G12 O88C SEC. 1004

BUILDING EXITING:	
MAX. FLOOR AREA ALLOWANCES PER OCCUPANT:	ASSEMBLY: 1 PER 50 SQ. FT. - EXERCISE (DANCE) ACCESSORY: 1 PER 300 SQ. FT. RESTROOMS NOT COUNTED O88C TABLE 1004.5
NUMBER OF REQUIRED EXITS:	(2) 1ST STORY (2) 2ND STORY O88C TABLE 1006.3.3 TRAVEL DIST. < 15'
MIN. EXIT WIDTH:	7.1' (0.2' PER OCC.) O88C SEC. 1005
MIN. CORRIDOR WIDTH:	44" TABLE 1003
EXIT ILLUMINATION:	SEE LIGHTING PLAN O88C SEC. 1003

FIRE DETECTION AND SUPPRESSION:	
FIRE ALARM AND DETECTION:	SEE SHEET G12 OFC SEC. 901
FIRE SUPPRESSION:	SEE SHEET G12 OFC SEC. 906
AUTOMATIC SPRINKLERS:	NS

DEFERRED SUBMITTALS:	
PLUMBING RISER DIAGRAM	
ELECTRICAL PANEL SCHEDULES / SINGLE-LINE DIAGRAM	
FIRE ALARM AND DETECTION	

VENTILATION REQUIREMENTS:	
ATTIC VENTILATION REQUIRED:	3,190 (net) / 150 = 21.3 SF O88C SEC. 1202.2
CRAWLSPACE VENTS REQUIRED:	NA. O88C SEC. 1202.4
NATURAL VENTILATION REQUIRED:	3,654 x 0.4 = 1462 SF OR MECHANICALLY VENTILATED O88C SEC. 1202.5

ENERGY REQUIREMENTS:	
ENERGY CODE:	2021 OREGON ENERGY EFFICIENCY CODE
CLIMATE ZONE:	ZONE - MARINE 4

HAZARDOUS MATERIALS:	
HAZARDOUS MATERIALS:	N/A

ACCESSIBILITY:	
SITE EXTERIOR ROUTES OF TRAVEL:	SEE SITE PLAN, SHEET A9.1 O88C SEC. 1104
BUILDING INTERIOR MEANS OF EGRESS:	SEE EXIT PLAN, SHEETS G12 O88C SEC. 1108
FACILITY ACCOMMODATION:	TOILETS, URINALS AND LAVS ELEVATORS OR PLATFORM LIFTS O88C SEC. 1110

PLUMBING REQUIREMENTS:	
OCCUPANT LOAD FACTOR:	152 TOTAL - ACCESSORY TO EXISTING THEATER O88C TABLE 2902.1
PROVIDED PLUMBING FIXTURES:	1 WC FOR 65 OCC., 3 TOILETS PROVIDED 1 LAV FOR 200 OCC., 3 LAVS PROVIDED
2 UNISEX RESTROOM:	2 WATER CLOSETS 2 LAVATORIES O88C SEC. 2902.3
DRINKING FOUNTAINS:	2 PROVIDED INCL. BOTTLE FILL. O88C SEC. 2902.1

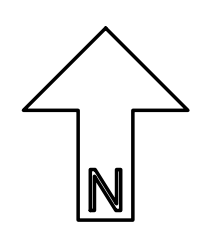
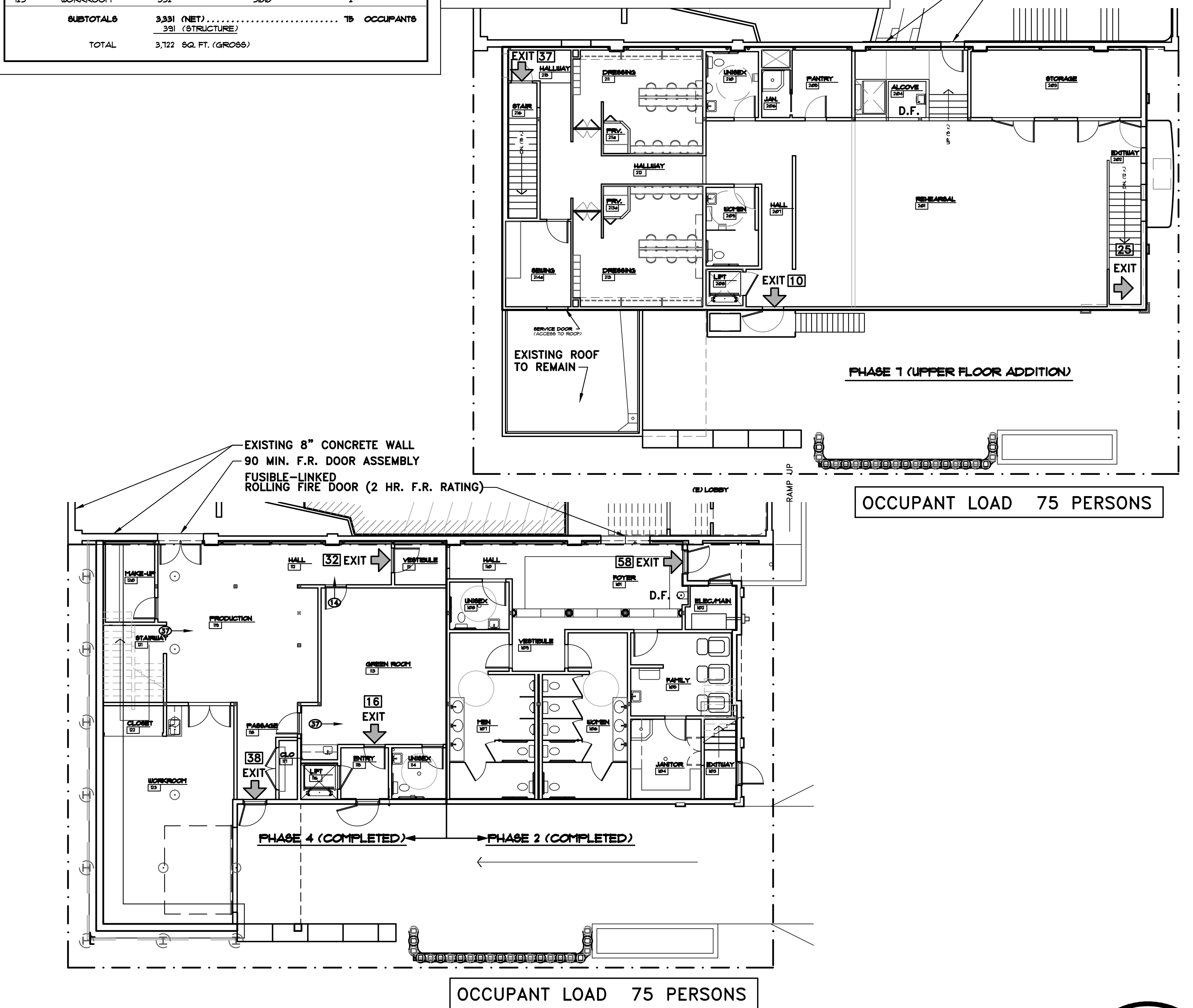
ROOF COVERINGS:	
TYPE / CLASSIFICATION:	MEMBRANE / CLASS B O88C TABLE 1505.1
	A8TM D4434 (PVC SGL-PLY) TABLE 1507.1.2

SPECIAL INSPECTION PROGRAM:	
STRUCTURAL WELDING PERIODIC VISUAL INSPECTION:	CERTIFIED SHOP WELDS, ONLY
OTHER:	NOT APPLICABLE
DEFINITION:	'AGENCY' INCLUDES TESTING LAB/ENGINEER BY CONTRACT

GENERAL NOTES:

ROOM AREAS AND OCCUPANT LOADS:				
LOWER FLOOR:				
MARK	NAME	AREA (SF)	SF/OCC.	OCCUPANTS
101	FOYER	295	15	20
102	ELEC.MAIN	42	NA	0
103	EXITWAY	50	300	1
104	JANITOR	112	300	1
105	FAMILY	166	50	4
106	WOMEN	261	NA	0
107	MEN	261	NA	0
108	UNISEX	57	NA	0
109	VESTIBULE	44	NA	0
110	HALL	44	NA	0
111	VESTIBULE	43	NA	0
112	HALL	77	NA	0
113	GREEN ROOM	421	15	29
114	UNISEX	56	NA	0
115	ENTRY	49	300	1
116	LIFT	27	NA	0
117	CLOSET	22	NA	0
118	PASSAGE	91	300	1
119	PRODUCTION	478	50	10
120	MAKE-UP	78	15	6
121	STAIRWAY	92	NA	0
122	CLOSET	33	NA	0
123	WORKROOM	532	300	2
SUBTOTALS		3,331 (NET)		75 OCCUPANTS
TOTAL		3,122 (STRUCTURE)		

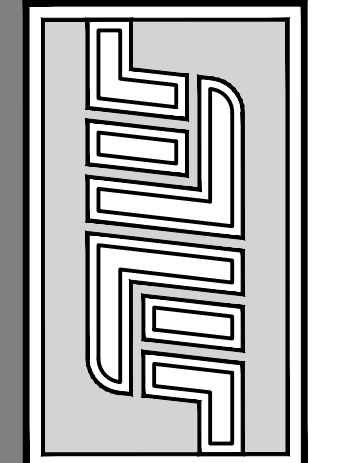
ROOM AREAS AND OCCUPANT LOADS:				
UPPER FLOOR:				
MARK	NAME	AREA (SF)	SF/OCC.	OCCUPANTS
201	REHEARSAL	1163	50	24
202	EXITWAY	118	NA	0
203	STORAGE	236	300	1
204	ALCOVE	160	50	4
205	PANTRY	81	300	1
206	JANITOR	28	300	1
207	HALL	198	50	4
208	LIFT	57	0	0
209	WOMEN	83	0	0
210	UNISEX	70	0	0
211	DRESSING	236	15	16
212	PRIVACY	15	15	1
213	HALLWAY	108	0	0
214	DRESSING	281	15	19
215	PRIVACY	15	15	1
216	SEWING	17	50	3
217	COSTUMES	489	300	2
218	HALLWAY	125	0	0
219	STAIRWAY	80	0	0
SUBTOTALS		3,165 (NET)		75 OCCUPANTS
TOTAL		236 (STRUCTURE)		



EXIT, OCCUPANCY AND FIRE/LIFE/SAFETY PLAN
SCALE: NTS

PHASE SEVEN ADDITION

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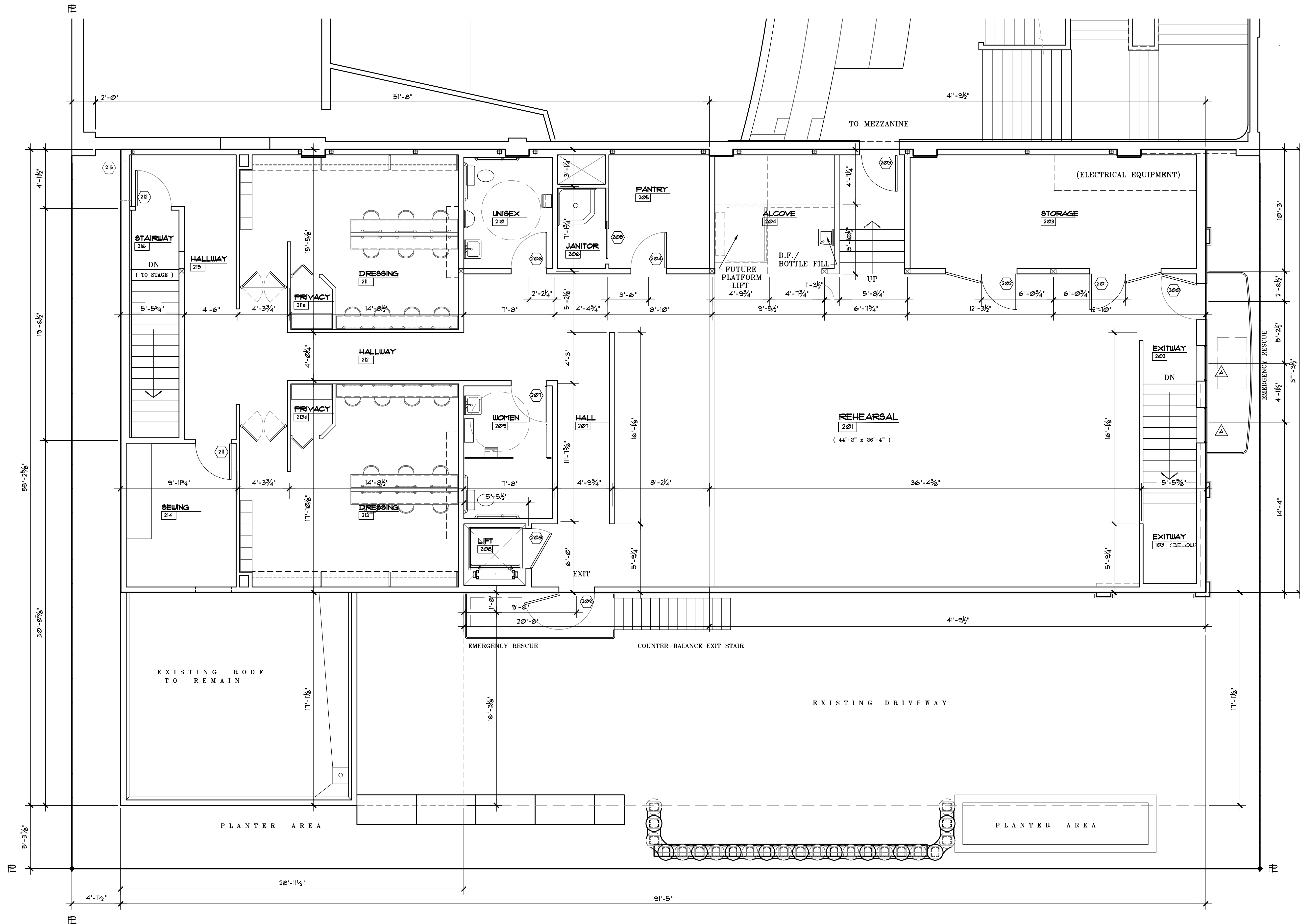
MCSWAIN - WOODS ARCHITECTURE
432 N. 4TH STREET, COOS BAY, OREGON
(541) 288-0618 (M) 503-708-3440 Email: stuart@mcswain-woods.com

SHEET TITLE:
BUILDING CODE ANALYSIS
FIRE, LIFE & SAFETY PLAN
OCCUPANT LOAD

DATES:	
PLAN REVIEW	6/25/23
ISSUE/BID	5/10/23
CONST. SET	
REVISION	8/30/23
REVISION	
REVISION	

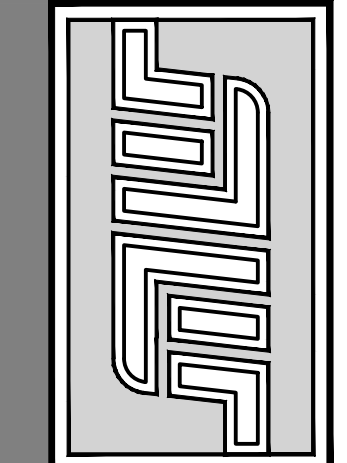
PROJECT #
1529.7

G1.2



SHERMAN AVENUE

UPPER FLOOR PLAN (NEW ADDITION)
 FINISH FLOOR ELEVATION = 61.60' TOTAL AREA (UPPER FLOOR ADDITION) = 3,920 SQ. FT. SCALE: 1/4" = 1'-0"



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SHEET TITLE:
 UPPER FLOOR PLAN
 I I

DATES:	FEB. 20, 2023
PLAN REVIEW	6/25/23
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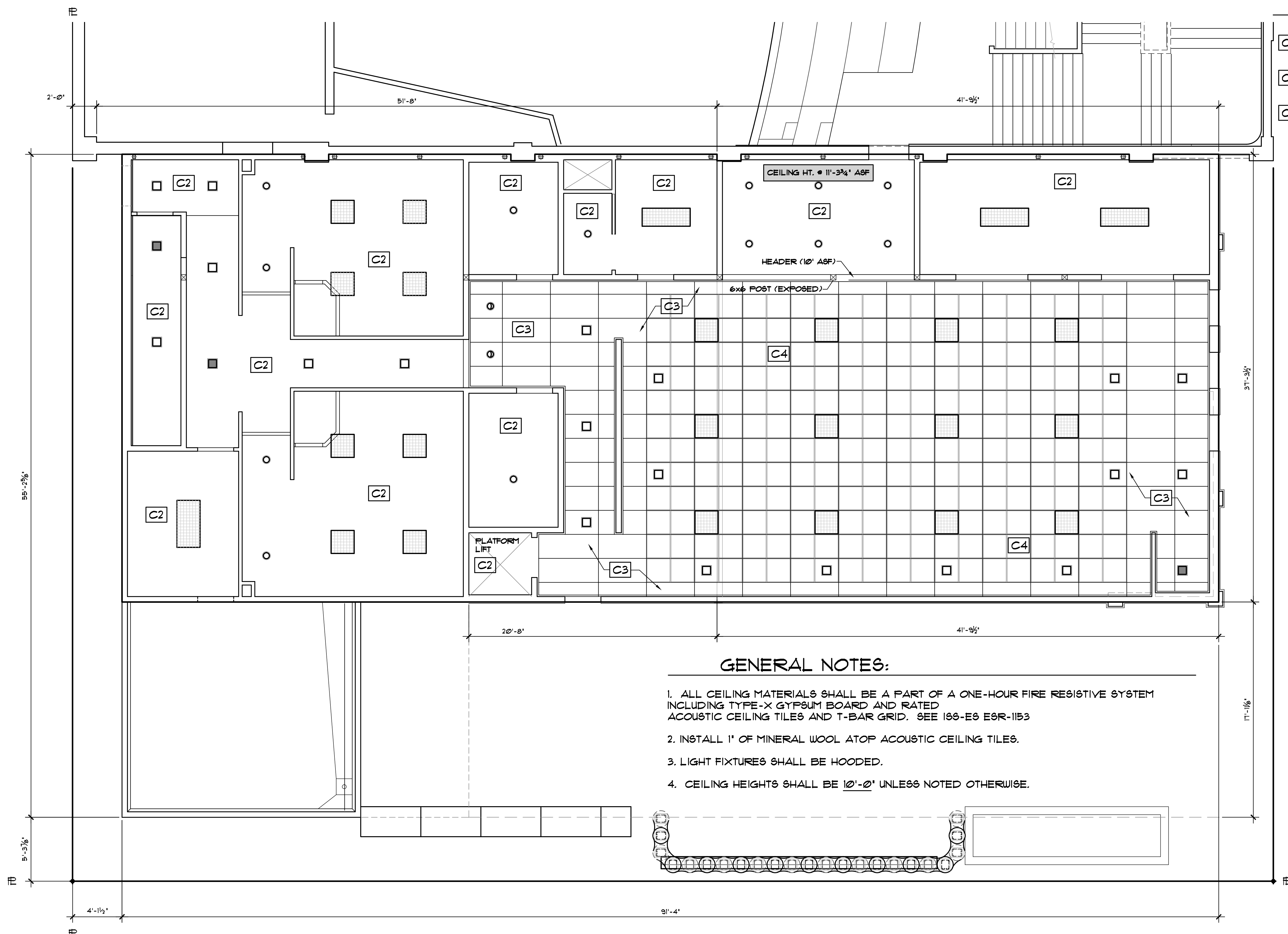
A1.1

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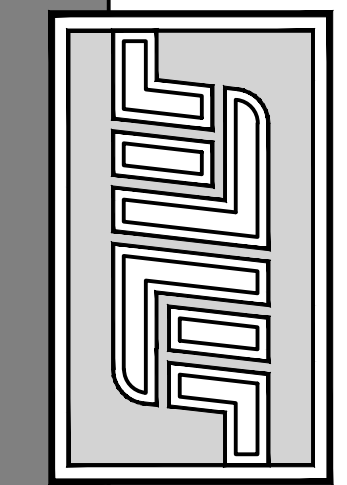
LEGEND

- C2 5/8" TYPE 'X' GYP. BD. - SUSPENDED ORANGE PEEL TEXTURE - PAINTED
- C3 48"x24" ACOUSTIC TILE / T-BAR GRID TYPE 1 TEXTURE, RATED
- C3 24"x24" ACOUSTIC TILE / T-BAR GRID TYPE 2 TEXTURE - REGULAR, RATED



GENERAL NOTES:

1. ALL CEILING MATERIALS SHALL BE A PART OF A ONE-HOUR FIRE RESISTIVE SYSTEM INCLUDING TYPE-X GYPSUM BOARD AND RATED ACOUSTIC CEILING TILES AND T-BAR GRID. SEE 155-ES ESR-1153
2. INSTALL 1" OF MINERAL WOOL ATOP ACOUSTIC CEILING TILES.
3. LIGHT FIXTURES SHALL BE HOODED.
4. CEILING HEIGHTS SHALL BE 10'-0" UNLESS NOTED OTHERWISE.



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 432 N. 4TH STREET, COOS BAY, OREGON
 (O) 541.269.0618 (M) 503.708.3440 Email architects@mcswain-woods.com

SHEET TITLE:
 REFLECTIVE CEILING PLAN

DATE:	FEB. 20, 2023
PLAN REVIEW	6/25/23
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REVISION	

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A1.2

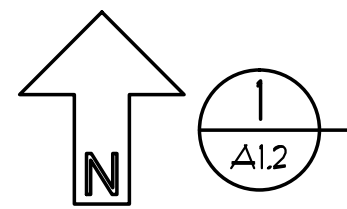


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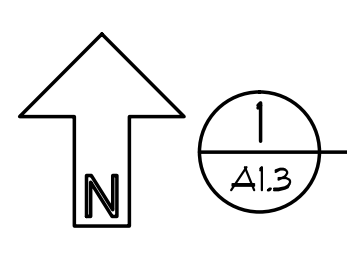
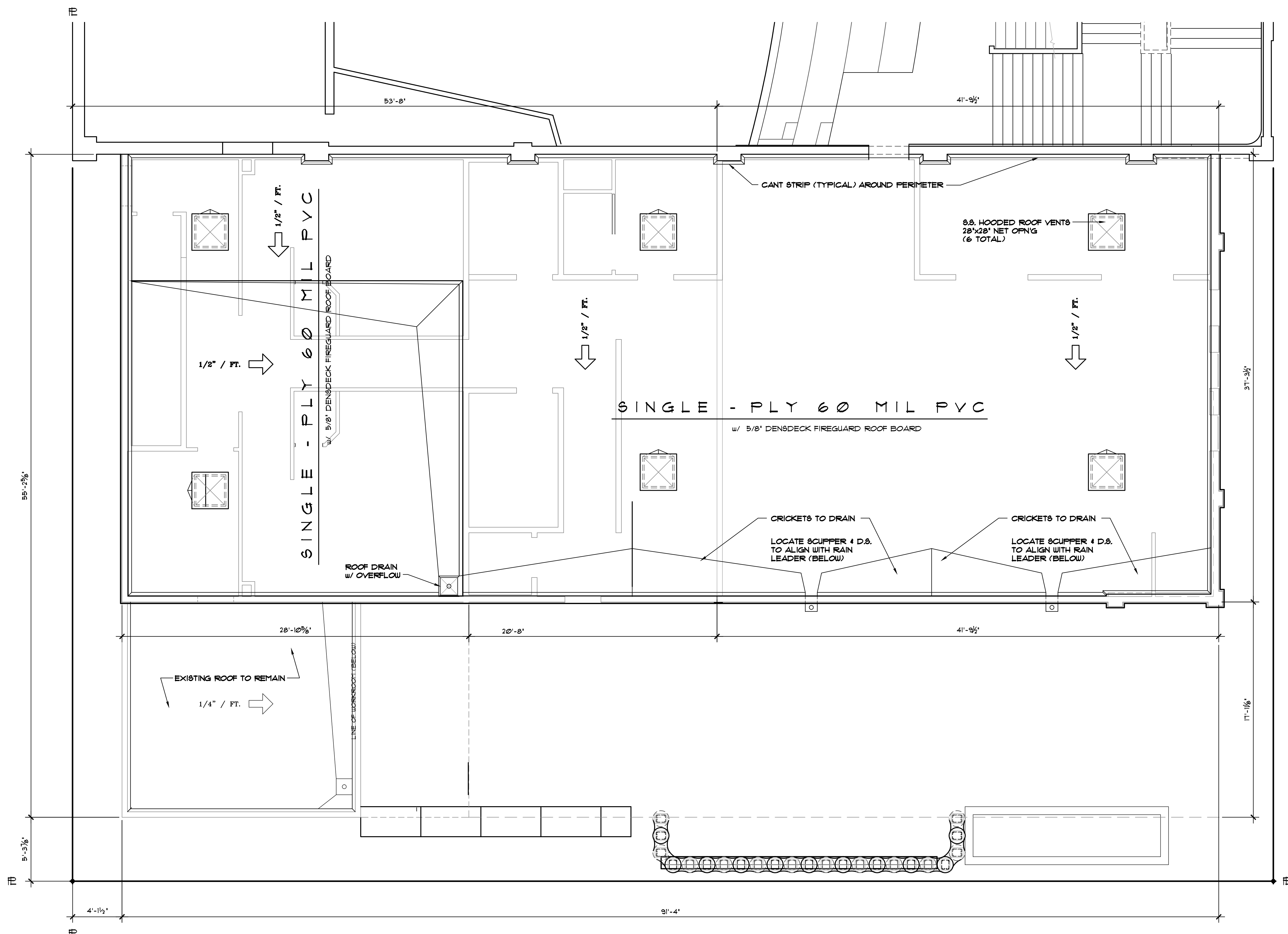
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REFLECTIVE CEILING PLAN

SCALE: 1/4" = 1'-0"



A12

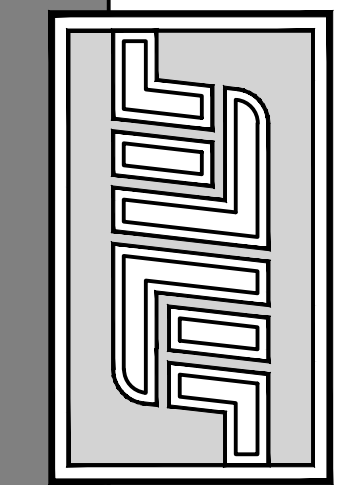


ROOF PLAN

SCALE: 1/4" = 1'-0"

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432 N. 4TH STREET, COOS BAY, OREGON
(O) 541.269.0618 (M) 503.708.3440 Email architects@mcswain-woods.com

SHEET TITLE:
ROOF PLAN

DATES:	FEB. 20, 2023
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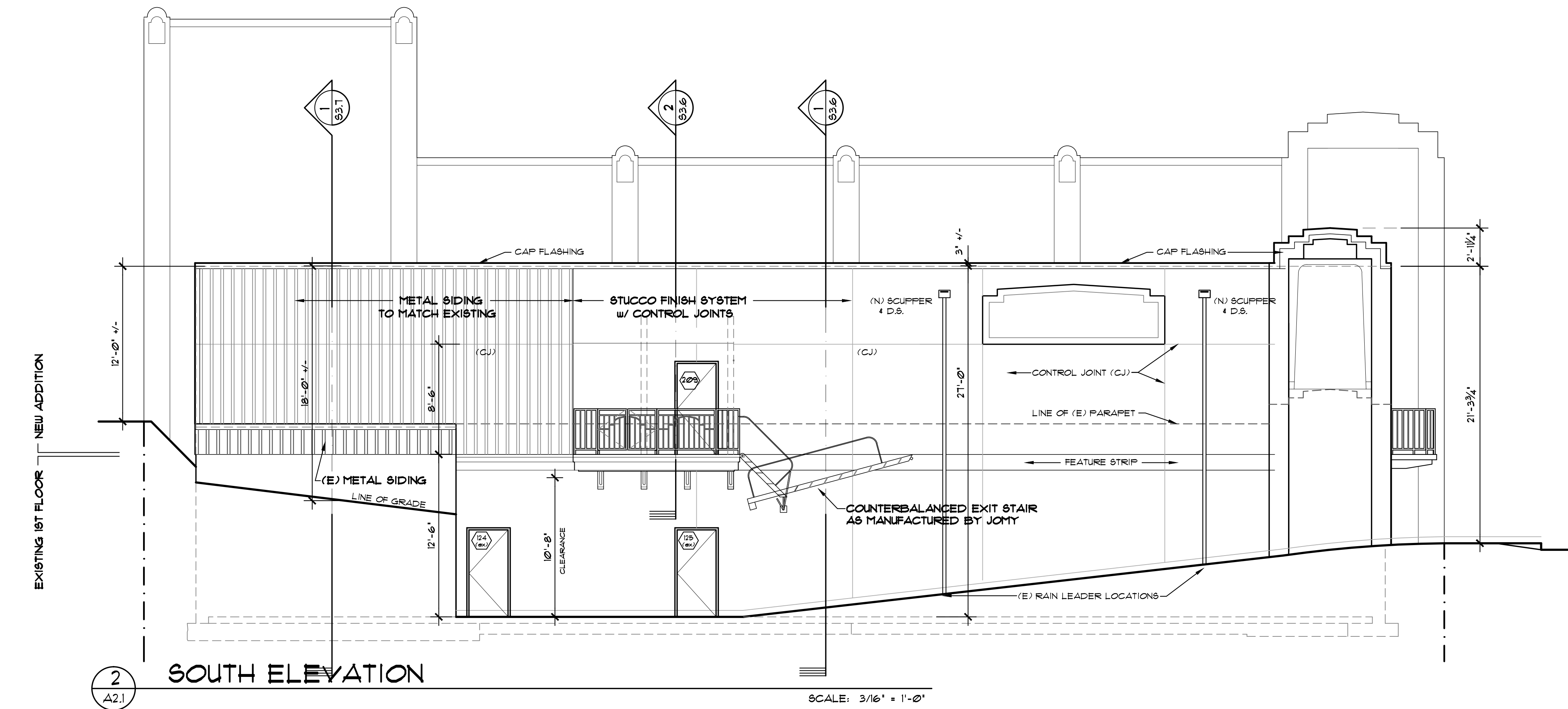
A1.3



EAST ELEVATION

SCALE: 3/16" = 1'-0"

1
A2.1



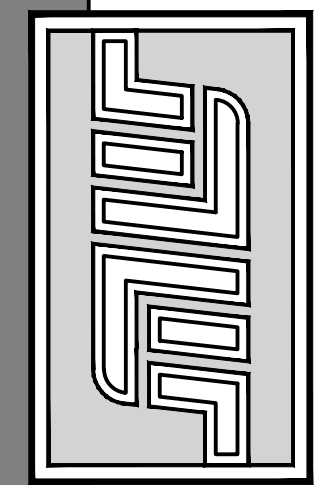
SOUTH ELEVATION

SCALE: 3/16" = 1'-0"

2
A2.1

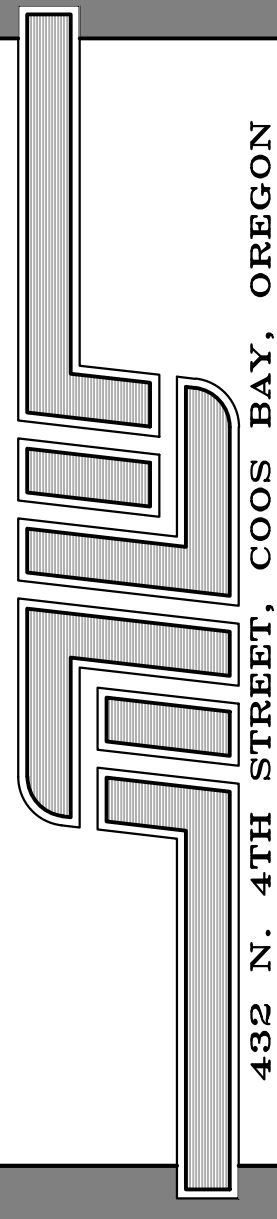
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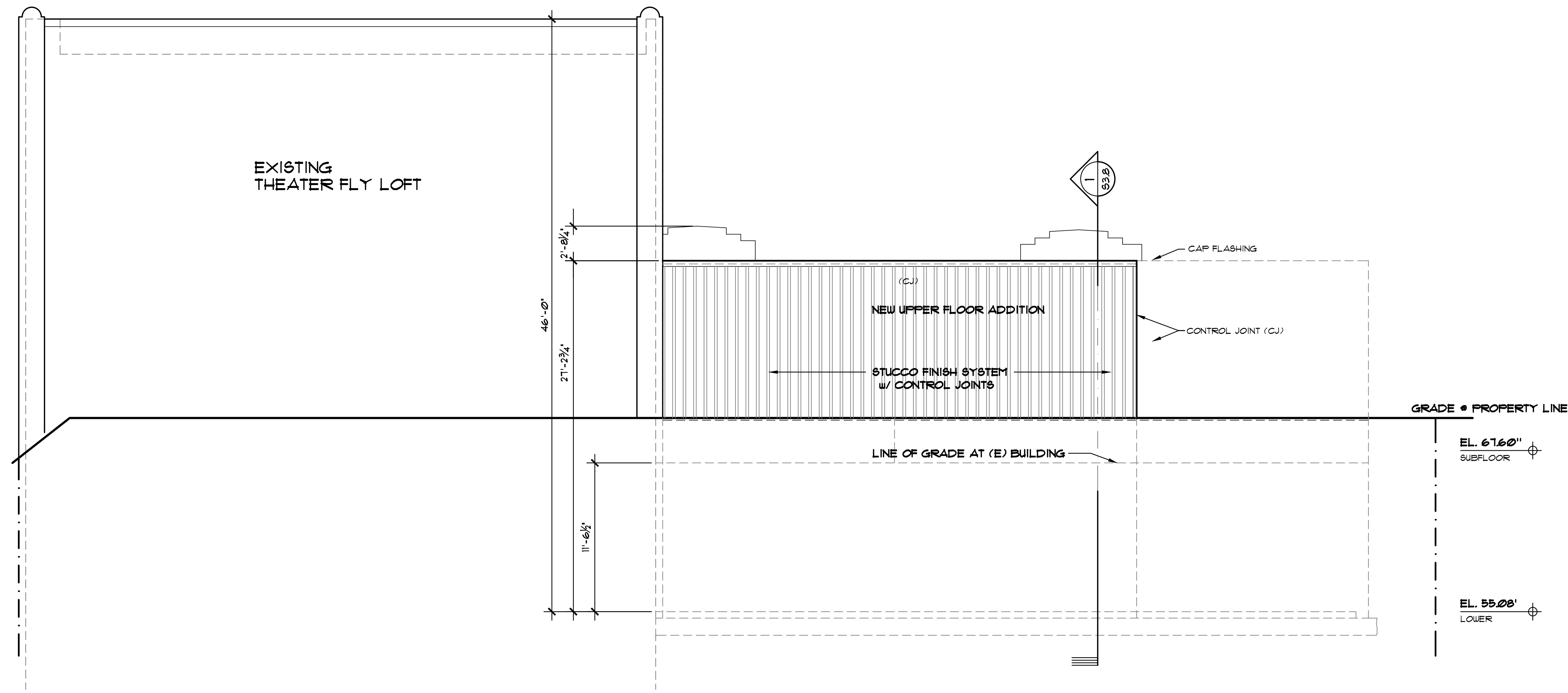
432 N. 4TH STREET, COOS BAY, OREGON
(O) 541.289.0618 (M) 503.708.3440 Email architects@mcswain-woods.com

SHEET TITLE:
EXTERIOR ELEVATIONS
EAST AND SOUTH

DATE:	FEB. 20, 2023
PLAN REVIEW:	6/25/23
ISSUE/BID:	5/10/23
CONST. SET:	
REVISION:	8/30/23
REVISION:	

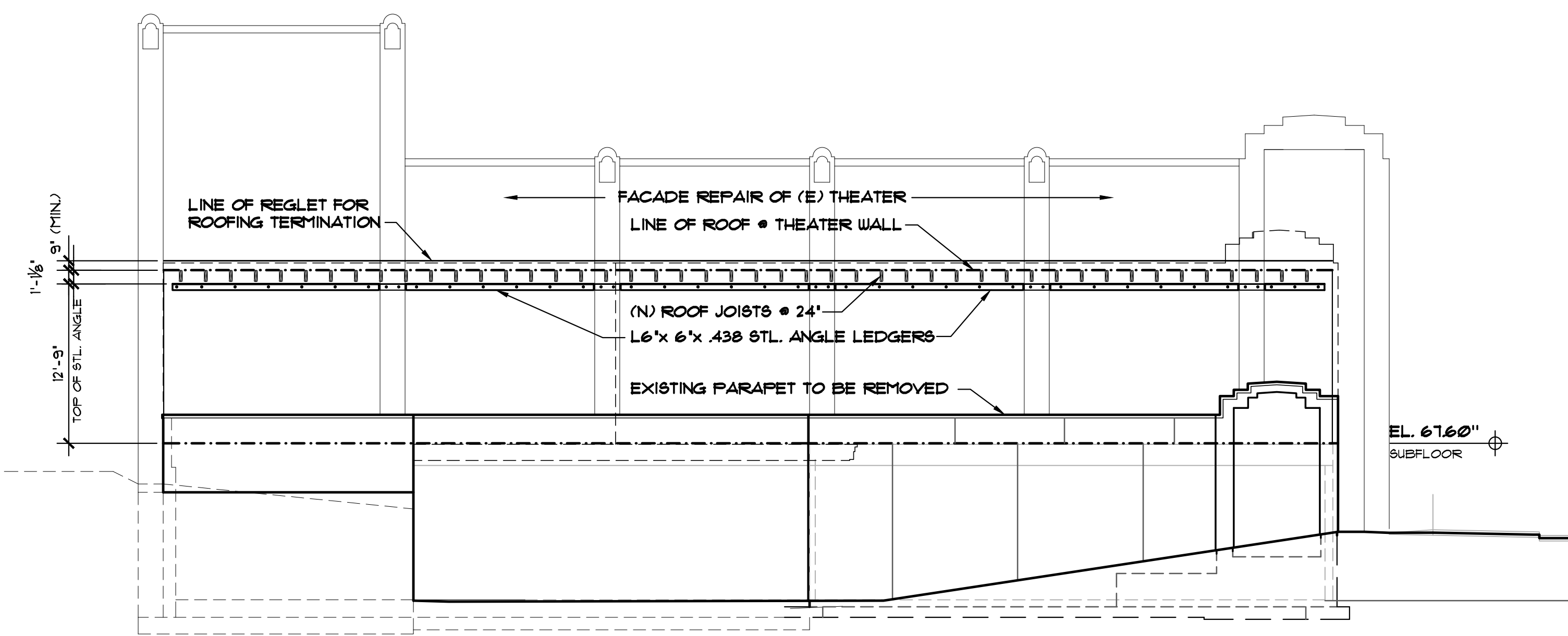
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A2.1



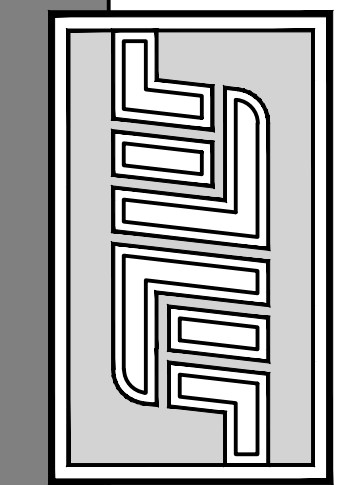
1 WEST ELEVATION

SCALE: 3/16" = 1'-0"



2 THEATER SOUTH WALL - REPAIR

SCALE: 1/8" = 1'-0"



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SHEET TITLE:
EXTERIOR ELEVATION - WEST

DATES:	FEB. 20, 2023
PLAN REVIEW	6/25/23
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A2.2

PHASE SEVEN ADDITION

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STRUCTURAL GENERAL NOTES

GENERAL REQUIREMENTS

1. Codes and Standards:

Design of new elements conform to the 2022 Oregon Structural Specialty Code (OSSC) based upon the 2021 International Building Code (IBC). All reference to other codes such as ACI, ASTM, etc. shall be the edition adopted by the OSSC.

2. Contractor's Responsibilities:

- The General Contractor (GC) is responsible for carrying out the requirements of these documents through the use of their own effort or that of subcontractors. The GC is responsible for all construction methods, techniques, sequencing, and safety procedures required to complete construction. All instructions contained in these documents are interpreted to be instructions to the GC and are the responsibility of the GC to fulfill.
- Verify existing conditions prior to proceeding with construction. Immediately bring discrepancies to the attention of the Architect of Record (AOR).
- Measure dimensions of any existing structures associated with the work and coordinate with required dimensions for new construction. Drawing scales are indicated for reference to assist with clarifying the work and providing proportions that resemble actual conditions. Do not measure drawings for construction. Use text dimensions provided. Request unknown dimensions from the Architect with sufficient lead time to prevent construction delay.
- Arrange for, and coordinate work by trades and suppliers. Facilitate required inspections, special inspections and tests specified herein, the OSSC and, by permit, where coverings or work conceals items or areas to be inspected prior to satisfactory approval, the GC is responsible for removal and replacement of coverings or work as necessary without cost to the Owner or Architect.
- Shop drawings are required for the following items:

SHOP DRAWING REQUIREMENTS

Drawings Req'd if Marked	ITEMS	Engineer's Stamp Req'd if Marked
[]	REINFORCING STEEL	[]
[]	STRUCTURAL STEEL	[]
[]	METAL DECK	[]
[]	MANUFACTURED WOOD AND GLU-LAM BEAMS	[]
[]	MANUFACTURED WOOD JOISTS	[]
[]	STEEL JOISTS AND GIRDERS	[]
[]	LIGHT-GAUGE METAL PLATE-CONNECTED WOOD TRUSSES	[]
[]	LIGHT-GAUGE STEEL TRUSSES	[]
[]	LIGHT-GAUGE STEEL FRAMING	[]
[]	PRE-CAST CONCRETE SLABS, BEAMS AND GIRDERS	[]
[]	POST-TENSIONED CONCRETE SYSTEM	[]

- Submit shop drawings for review, for each material indicated, prior to construction. Shop drawings do not replace or supersede the requirements of the structural drawings. Special inspection shall be based on the structural drawings. Any discrepancy between the shop drawings and structural drawings shall be reported to the AOR by the GC and by the special inspector for items that require inspection.
- This structure must be fully braced for wind and seismic loads during construction (See Item 2A, above). Contractor provided bracing must remain in place until the permanent lateral force resisting system of the structure is completed.
- Where/If construction occurs around existing structures, protect existing footings from being undermined. Provide engineered shoring as necessary to protect existing structures. Repair damage, where caused by construction activity, to existing structures.
- Install items manufactured or supplied by others per the manufacturer's specifications.
- Where details of construction are not explicitly shown, provide materials and construction of the same type and character as that of similar conditions used on the project. The actual details used shall be submitted to the AOR for approval prior to ordering materials or beginning construction.
- Any conflict or discrepancy shall be brought to the attention of the Architect of Record for clarification and resolution prior to ordering materials or beginning construction.

3. General Contractor Warranty Requirements:

- By acceptance of this project, the General Contractor (GC) warrants that:
 - The GC and subcontractors hired by the GC have carefully and thoroughly reviewed the drawings and structural notes and have found them complete and free from ambiguities and sufficient for purpose intended - further that,
 - The GC has carefully examined the work site and that from those investigations is satisfied as to the nature and location of the work, as to the character, quality, quantities of material, and difficulties to be encountered, as to the extent of equipment and other facilities needed for the performance of the work, and as to the general and local conditions, and other items which may affect the work or its performance - further that,
 - The GC and all workers are experienced in the type of construction represented by the drawings and other documents further that,
 - Neither the GC or their employees, agents, intended suppliers, or subcontractors have relied upon verbal representations allegedly authorized or unauthorized from the Owner or their employees or agents, including the Architect of Record.
- Bring discrepancies to the attention of the Architect of Record at least 10 days prior to the close of bid.
- The GC warrants that the construction cost for items not brought to the attention of the AOR prior to bid, but that require clarification or AOR assistance during construction, will be covered by the bid amount agreed to by the GC. See Item 2G, above.

4. Design Loads:

- Roof Loads**
Total Dead Load _____ = 20 psf
Live Loads (now) _____ = 20 psf
- Floor Loads**
Total Dead Load _____ = 20 psf
Live Load (future) _____ = 100 psf
- Flood Load**
not applicable: above the 100 year flood plain
- Wave Load**
not applicable: protected by sheet-pile bulkhead
- Impact Load**
not applicable: protected by sheet-pile bulkhead
- Rain Load**
flat roof: add 2.17 pif along South parapet
- Wind Loads**
3-sec. gust @ 100 mph - basic wind @ Vasd = 95 mph
Imp Cat. C
Importance factor (II) @ 1.0
- Earthquake Loads**
Site class D - Design cat. D
Shear factor V = 0.154W

5. Equipment and Non-structural Components:

- Determine actual equipment and non-structural component weights, locations, and sizes supplied for this project. Notify Architect if weights exceed allowances noted on the structural plans and if equipment footprints are reduced from that shown on the plans.
- For equipment and non-structural components weighing more than 400 lbs., prepare and submit seismic anchorage calculations and details sealed and signed by a Professional Engineer registered in the State of Oregon.
- Coordinate between subcontractors to insure that:
 - Additional secondary framing is provided as required. Design of equipment and secondary framing by the vendor's engineer shall conform to OSSC requirements.
 - Edges of decking are supported at all openings. Deck penetrations are made and reinforced to conform to deck vendor's reinforcing recommendations.
 - Openings, penetrations and accessories are located to avoid interference with structural elements.
 - Fire-engineered elements are designed to support all weights and forces.
- Brace equipment and accessories per SMACNA Seismic Restraint Manual. Unusual ductwork, piping, or conduit configurations that fall outside of SMACNA minimum limits should be securely restrained to prevent movement.
- Brace suspended ceilings per UBC Standard 25-2 or ICBO Report No. 4071.

INSPECTION AND TESTING

1. Construction:

- Construction will be inspected as required by the OSSC and as described in the Special Inspection Schedule. Special inspections and/or structural observations do not replace OSSC Section 1704 inspections by the building inspector.
- Items noted as requiring special inspection in accordance with OSSC Chapter 17 shall be performed by a qualified person who can demonstrate certification for the particular type of construction being inspected. The Special Inspection Agency shall be independent from contractors and suppliers related to the job. The Architect (AOR) retains the right to reject, for any reason, the agency chosen for the job.
- The Special Inspector shall not be hired by the Contractor (GC). The Owner or an independent agent of the Owner shall hire the Special Inspector.
- Special inspection is required per the Special Inspection Program, and/or as noted in the Project Manual or on the Drawings and shall be performed in accordance with Section 1701 of the OSSC.

2. Site Preparation

- The site has been excavated to approximately subgrade for the new slab. Remove loose material to a level at least twelve inches below the existing grade. Roll three passes over the building area with a heavy vibratory roller. Extend rolled area at least one foot outside the perimeter of the footing line. Over-excavate any areas that the Architect determines to exhibit excessive deflection. Place structural fill to replace removed material per the following paragraph:

3. Structural Fill and Compaction

- Place structural fill and/or backfill after removal of forms, screeds, or other wood debris and material subject to decay or corrosion. Use only materials approved by the Geotechnical Engineer or Architect for fill. Limit fill to clean, granular material placed in loose eight (8) inch lifts and compacted to at least 95% of its maximum dry density as determined by ASTM D1557 compaction test procedures. Verification of compaction will be done by random field density tests per the Special Inspection Schedule. Use light-weight hand operated equipment to compact fill within six (6) feet behind walls. For any site, follow the recommendations of the Geotechnical Report in place of requirements specified herein.

FOUNDATIONS

1. General:

- Specified design parameters:
 - Soil bearing (Foundations)
Dead + Live _____ 2,000 psf
Dead + Live + Eq/W _____ 2,500 psf
 - Retaining Walls
Unrestrained Walls (active) _____ 35 psf
Restrained Walls (at-rest) _____ 60 psf
Passive Earth Pressure _____ 150 psf
Coefficient of Friction _____
Not applicable for this project.
- Contact, Architect, for a site visit, after site excavation but prior to any foundation construction. The site visit is to verify adequacy of actual soil conditions and to determine whether Architect visit is necessary and for special inspections required to satisfy OSSC related to soils, structural fill and/or backfill.
- Place footings and slabs as directed by the soils report over the drawings. Footings may be placed on firm original material as approved by the Architect during a site visit.

- Center footings on walls or columns above unless noted otherwise on plans or sections of the Drawings.
- Place backfill behind walls after wall material obtains its design strength and no sooner than seven (7) days after placement of concrete or grout. For retaining walls, use only backfill material that is free draining granular fill free of fines, silt or clay and approved by the Architect. Install and compact backfill in conformance with the soils report or the specifications, herein. The GC is responsible for bracing walls during the backfill operations. At Contractor's expense, replace walls damaged or displaced by improper backfilling operations.

REINFORCED CONCRETE

1. Codes, Standards and Specifications:

- The OSSC (Building Code) governs in the event an ACI standard specification requirement conflicts with a Building Code requirement.
 - Building Code - OSSC Chapter 19
 - Concrete reinforcement - ACI 318
 - Welding - AWS D1.4
 - Handling, Placing & Construction - ACI 301
 - Tolerances - ACI 117
 - Curing - ACI 308
- Provide structural concrete meeting the requirements specified in the Table, herein.
- Contractor is responsible for concrete transporting and placing methods, and specification of supplementary concrete mix requirements (e.g., admixtures and properties) associated with construction issues like weather, transport, placement, workability, finishing and form/shoring removal.
- Sample and test concrete at the truck discharge point for quality control purposes. When the Contractor elects to pump concrete, mix design information should include supplemental air content and slump at the placement end of the pipe.

CONCRETE CLASS APPLICATION TABLE

Location/ Application	Specified Compressive Strength (f'c) (See Note 1)	Normal Max. Size Aggregate (See Note 1)	Maximum W/C Ratio (See Note 1)	Maximum Slump (See Note 2)	Minimum Pump (See Note 3)	Minimum Air Content (See Note 4)	Class Concrete
Footings, Beams and General Slabs (where specified)	3000 psi	3/4 in.	0.54	5 in.	5 in.	3% max. (See Note 4)	3000-3/4
Walls (where specified)	3000 psi	3/4 in.	0.52	200	5 in.	3% max. (See Note 4)	3000-3/4F
Slabs, Topping slabs	4000 psi	3/8 in.	0.50	200	5 in.	3% max. (See Note 4)	4000-3/8F
Beams, Sills and Columns	4000 psi	3/4 in.	0.50	200	5 in.	3% max. (See Note 4)	4000-3/4
Walls (where specified)	4000 psi	3/4 in.	0.45	210	4 in.	4% min. 6% max.	4000-3/4AE

Table Notes:

- Maximum W/C ratio is in addition to minimum compressive strength and maximum water content requirements. Limit fly ash to 20% of total cementitious material by weight.
- When specified, limit water including site-added water allowance and aggregate-free moisture to amount specified.
- Maximum slump specified is at the point of placement. Where point of placement differs from the discharge chute of the mixer (e.g., pumped) provide mix design slump ranges at both the mixer chute discharge point and at the point of placement.
- Do not add air entrainment agent.

2. Reinforcing Steel (Rebar):

- Fabricate, detail, and place in accordance with the Building Code supplemented by the following: ACI 318 Details and Detailing of Concrete Reinforcement and AWS D1.4 Structural Welding Code - Reinforcing Steel.
- Materials, unless noted otherwise (UNO):
Deformed bar reinforcement - ASTM A615 Grade 60
Welded deformed bar reinforcement - ASTM A106 Grade 60
Welded wire fabric (flat sheets) - ASTM A185
- See Lap Splice Length Table for minimum lap splice lengths and spacing requirements. Locate beam and column splices where specified. Stagger wall and flatwork splices as required to minimize bar congestion.
- Do not weld reinforcing steel unless specified by design or without authorization by the Architect. Where welding is specified, use A106 bars with matching filler metal requirements per AWS D1.4.
- Do not tack-weld reinforcing steel.
- Support reinforcement with approved chairs, spacers, or ties. Space and locate bolsters and ties to resist damage to the rebar caused by construction activities.
- Support concrete slab-on-grade reinforcing steel using prefabricated chairs with sand pads, or precast concrete dobbies with length and width not narrower than the required height. Use dobbies with embedded tie wire.

3. Reinforcing Protection (Cover):

- Concrete deposited against the earth. _____ 3 inches (2 1/2" underslab)
- Concrete formed surfaces
Exposed to ground or weather
#5 and smaller bar _____ 1 1/2 inches
#6 and larger bar _____ 2 inches
- Concrete formed surfaces
Not exposed to ground or weather
Slabs, walls & joists, #1 and smaller _____ 3/4 inches
Beams and columns _____ 1 1/2 inches

LAP SPICE LENGTH (Ls) TABLE (ACI 318 Class B)

BAR SIZE (d)	CONCRETE STRENGTH (f'c)		
	3000 psi / 3000 psi	4000 psi	4000 psi
	TOP (N) (2)	OTHER (N) (3)	OTHER (N) (3)
#3	28	22	24
#4	31	25	28
#5	41	36	40
#6	56	43	48
#7		50	54
#8		61	62
#9		71	70

Table Notes:

- Space bars at least 2-bar diameters clear between adjacent bars.
- 'Top' bars are horizontal bars with more than 12' of fresh concrete below the splice.
- 'Other' bars are vertical bars and bars with less than 12' of fresh concrete below the splice.

4. Construction Joints (CJ):

- Construction joints shall be located where specified or as approved by the Architect.
- Construction joints shall be made to transfer shear across the construction joint by intentionally roughening the surface to full amplitude and spacing of approximately 1/4 inch in two directions. Intentional roughening may be made while the initial placement is still plastic.
- Keyed construction joints may be used only when explicitly detailed.

B. Walls: Not used in this Phase Two

- Place single layer vertical bars at the center of the wall thickness unless noted otherwise. Place double mats as detailed.
- Provide (2) #5 trim bars around all openings and recesses unless noted otherwise. Extend trim bars at least the development length 'Ld' (see Table) beyond the corner of the openings, extend as far as possible and terminate the bar with a standard hook.
- Provide corner bars of same size and spacing as horizontal bars and lap splice (Ls) to horizontal bars.

6. Structural Embedded Items:

- Secure all structural anchor rods (bolts) in place and have inspected prior to delivery of concrete. Insertion of embedded items into plastic concrete is prohibited.
- Locate structural anchor bolts within tolerances consistent with structural steel erection tolerances.
- Embedded plates are to be installed flush with concrete surfaces unless noted otherwise and secured against movement during placement and consolidation.
- Anchor rods (Bolts) - See Structural Steel Note 4.

7. Conduit, Pipes & Sleeves (Non-structural Embedments):

- Conduit, pipes, sleeves and other non-structural items embedded in structural concrete shall be of material not harmful to concrete, and shall be limited in size, quantity, spacing and configuration, such that they collectively do not significantly impair the strength and integrity of the structure.
- GC is responsible for the collective impact of conduit, pipe, sleeves and other non-structural items embedded in and/or penetrating structural concrete. The GC is also responsible for coordinating the placement of all items embedded in structural concrete.
- Conduit, pipes and sleeves embedded in structural concrete require approval of the Architect. Embedment of conduit, pipes and sleeves is acceptable without advanced approval only if all of the conditions noted below are met. Submit all exceptions to the Architect for approval in advance.
 - Do not place aluminum items in concrete. The Architect may approve aluminum items that are coated or covered to prevent adverse reactions provided that the GC submits substantiating data.
 - Pipes embedded are low pressure (<50 psi) and not hot (<150 °F). Do not energize pipes until the concrete reaches its design strength. Design pipes and fittings to resist the effects of the material pressure, and temperature to which they will be subjected without reliance on the concrete.
 - Columns. Limit embedded items to less than 4% of the cross-sectional area of columns. Place embedded items in the center third of its depth and width (or, diameter). Limit penetration of embedded items to the center third of the column and make the penetration at a right angle to the column face.
 - Beams. Place embedded items in or penetrate structural concrete beams (or, girders) in the middle third of the element's depth and limit embedded items to a maximum section loss of 10% of the beam's cross-section area.
 - Walls. Limit embedded items in or penetrating structural concrete walls to 4% of the wall's cross-section area. Do not place embedded items in the 15% of the wall length closest to the wall's ends or corners. Limit embedded items' greatest outside dimension to no more than three diameters or widths center to center. Locate embedded items within the center third of the wall's thickness except where the item enters or exits the wall at a right angle.
- Structural Slabs. Limit embedded items size in or penetrating structural slabs to less than one-third of the slab's overall depth. Space items at least six (6) diameters center-to-center and locate in the center third of the slab, except where the item enters the slab at a right angle. Note: Slab-on-grade floors are exempt from this requirement.
- Center-to-center spacing of dissimilar size items shall be the sum of 1 1/2 diameters of the smaller item and 1 1/2 diameters of the larger item.
- Where embedded items cross within a wall or elevated slab, place at right angles.
- Composite concrete-metal deck slabs. Block-out vertical penetrations. Vertical penetration block-outs smaller than six (6) inches in diameter do not require reinforcement provided that no more than one web of the metal deck is cut and that penetrations are spaced on-center at least 32' parallel to deck flutes. Reinforce larger penetrations or closely spaced penetrations to restore structural capacity.
- Do not cut or terminate wall reinforcing steel or bend rebar without approval.
- Locate embedded items penetrating concrete exposed to weather or in contact with the ground or fill providing at least 1 1/2 inches of clearance between the items and reinforcing steel.

8. Placement and Curing:

- Convey, deposit and consolidate concrete in accordance with ACI 301. Mechanical vibration is mandatory for all elements 12' and deeper.
- Make saw-cut or cooled shrinkage control joints in flatwork as soon as possible and on the same day as concrete placement. Fill joints as directed by the Architect.
- Protect curing concrete against cold weather effects. Maintain concrete above 50 F.
- Protect curing concrete against hot weather effects. Keep concrete continuously moist when air temperature exceeds or are expected to exceed 85 F.
- Moist cure all concrete for at least the first seven (7) days after placement. During cold weather work, maintain cure for fourteen (14) days.
- Cure flatwork using approved water curing, plastic film or reinforced paper methods. Cure vertical surfaces using the flatwork methods or approved liquid membrane-forming compounds following form removal. At the end of the curing period, discontinue wetting and allow concrete to dry slowly.
- Submit curing plans specifying methods and materials in advance to the Architect.

9. Non-shrink Grout:

- Provide non-metallic, non-shrink grout conforming to requirements of ASTM C1071, Type B or C, with an ASTM C-109 compressive strength of 5000 psi in seven (7) days. Place and cure following manufacturer's instructions.

CONCRETE UNIT MASONRY

1. Codes Standards and Specifications:

- OSSC (Building Code) governs in the event an NCMCA standard specification requirement conflicts with a Building Code requirement.
 - Building Code - OSSC Chapter 21
 - CMU reinforcement - NMCA TEK 12-4B
 - Handling, Placing & Construction - NMCA TEK 3-1A
- Contractor is responsible for CMU transporting and placing methods and specification of supplementary mortar and grout mix requirements associated with construction issues like weather, transport, placement, workability, finishing and cleaning.

2. Concrete Unit Masonry:

- Meet requirements of ASTM C-90, Type I, moisture control units, normal weight, load-bearing classification.
 - Outside corners shall be square-edged.
 - Use special shapes for jambs, bonding, control joints, etc.
 - Uniform color and texture with blended scored and ground-face units selected by owner.
- See drawings for size requirements.

3. Masonry Mortaring:

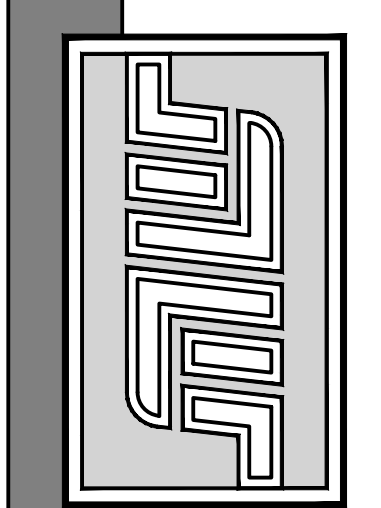
- Performance = Minimum compressive strength at 28 days shall be 2500 psi.
 - Type 5.
 - Use: Portland Cement - ASTM C150-05
Hydrated Lime - ASTM C207, Type 5
Aggregate - ASTM C144.
 - Parts by weight: FC @ 94 lbs. HL @ 20 lbs.
Dry sand @ 360-480 lbs.
 - Parts by volume: FC @ 1, HL @ 1, Damp Sand @ 2-1/4 -3 times sum of volumes of cement and lime.

4. Masonry Grouting:

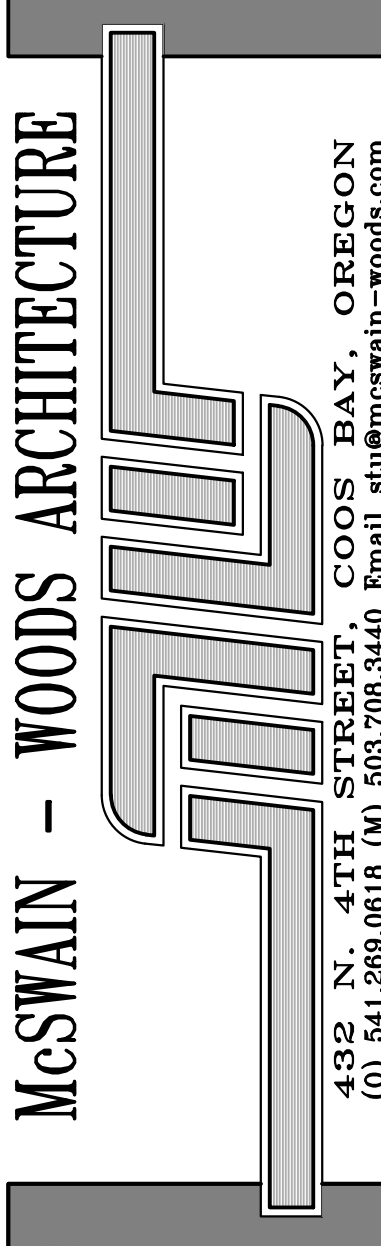
- Performance = Minimum compressive strength at 28 days shall be 4000 psi.
 - Use ready-mixed concrete - ASTM C94.05
or
Use: Portland Cement - ASTM C150-Type II Low Alkali
Hydrated Lime - ASTM C207, Type 5
Aggregate - ASTM C404 Table 1
Fine Grade, size 2
Coarse Grade, Size 8
 - Use fine grout for cavities 2 inches and smaller in smaller dimensions. Use coarse grout for cavities greater than 2 in. in smaller dimension.
 - Proportions by Volume:
Water: Slump between 8-10
Fine Grout: FC @ 1 cu. ft.
HL @ 1/2 cu. ft.
Sand @ 2 1/2 - 3 cu. ft.
Coarse Grout: FC @ 1 cu. ft.
HL @ 1/2 cu. ft.
Sand @ 2 1/2 - 3 cu. ft.
Pea Gravel @ 1 - 2 cu. ft.

4. Masonry Reinforcing Bars:

- Place rebar sizes and shapes where shown on Drawings. See sheet S21 for typical CMU wall sections and elevations



PROJECT TITLE:
YOUTH THEATRE / PROGRAM CENTER
LIBERTY THEATRE - PH. 7 ADDITION
2100 SHERMAN AVE.
NORTH BEND, OREGON 97459



SHEET TITLE:
STRUCTURAL GENERAL NOTES

DATES:
FEB. 20, 2023
PLAN REVIEW 6/25/23
ISSUE/BID 5/10/23
CONST. SET _____
REVISION A 8/30/23
REVISION B _____
REVISION C _____
PROJECT #
1529.7
S0.1

PHASE SEVEN ADDITION

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STRUCTURAL GENERAL NOTES (continued)

STRUCTURAL STEEL

1. Codes, Standards and Specifications:

- Detail, fabricate and erect in accordance with the following as adopted by the OSBC
 - A. 1989 AISC 'Specifications for Structural Steel Buildings'
 - B. 2000 AISC 'Code of Standard Practice for Steel Buildings and Bridges'
 - C. AWS D11 Structural Welding Code - Steel

2. Structural Steel Material (UNO)

- W, WT shapes
 - ASTM A992
 - or, ASTM B12, Grade 50
- HSS tubes
 - ASTM 500, Grade B
- Pipe
 - ASTM A53, Grade B
- Angles and other shapes
 - ASTM A36
- Plate and Bar stock
 - ASTM A36

3. Bolts

- A. High-strength bolts (HSS) shall be ASTM A325N with threads not excluded from the shear plane unless designated A325X. Heavy hex nuts shall be ASTM A563 with ASTM F436 hardened washers, grade and finish to match bolts.
- B. HSS not specified as 'slip critical', and bearing connections should be at least 'brng tight'.
- C. ASTM F1852 twist-off tensioned controlled bolt assemblies may be used where A325 bolts are specified.
- D. F193 compressible-washer-type direct-tension indicators or alternative load indicator bolts equivalent in strength shall be used for all bolted joints noted as slip-critical (SC) or tension loaded.

- E. All other bolts not designated as high strength or specified to be machine bolts (MB) shall be:
 - ASTM A307, Grade A bolts
 - ASTM A563, Grade A hex nuts
 - ASTM F844, steel washers.
- F. Exterior bolts exposed to weather shall be zinc-coated in accordance with ASTM A153, Class C.

4. Anchor Rods (Anchor Bolts)

- A. Anchor rods embedded in concrete shall be ASTM F1554 Grade 36 with matching heavy hex nuts unless noted otherwise.
- B. Locate, place and secure anchor rods within erection tolerances defined by the AISC Standard Practice Section 7.5.
- C. Provide sufficient rod projection above the column base plate and plate washers (if any) to allow for full engagement of a matching heavy hex nut and washer.
- D. Anchor rods are not designed as compression members. Grout all base plates as soon as structural frame is plumbed and before significant load is applied to the base plate.
- E. All-thread anchor rods where specified, are continuously threaded bar with compatible ASTM A108 hex nuts conforming to ASTM A615 Grade 75, Grade 75 All-thread rebar, or approved.

5. Welds:

- A. Weld Procedure Specifications (WPS) shall be prepared and submitted in accordance with AWS D11 for all welding subjected to Special Inspection requirements.
- B. Welding electrodes shall conform to AWS D11 Table defining pre-qualified base metal/filler metal combinations for matching strength.
- C. End return fillet welds 2X nominal weld size wherever practical, unless noted otherwise.
- D. Minimum fillet weld size, See AISC Table J2.4 reproduced below:

Material Thickness of Thicker Part Joined in Inches, Minimum	Minimum Leg Size of Fillet Weld in Inches, (single-pass welds, only)
To 1/4 Inclusive	3/16
Over 1/4 to 1/2	1/4 - or, as noted on Drawings
Over 1/2 to 3/4	5/16 - or, as noted on Drawings
Over 3/4	1/2

- E. Shop welds may be field welded and field welds may be shop welded unless noted otherwise.

6. Headed Studs: Not used in Phase Two

Headed studs shall be TRW Nelson Type 83L or H4L with flared ends or approved equal (AWS D11 Table 7.1 Type B). Provide appropriate ferrules and required accessories to accomplish the required automatic weld. Specified length is the nominal after-welded length (AWL). Studs shall be welded with automatically-timed stud welding equipment per AWS D11 Section 7.5.

A. Stud qualification shall conform to AWS D11 Section 7.6

B. Stud production control testing shall conform to AWS D11 Section 7.7

7. Coatings:

- A. Steel work concealed by interior building finish or in contact with concrete need not be painted unless specified otherwise. In such cases, clean with wire brushing, degreasing pressure washing, and/or other methods to remove slag, dirt, oil/grease as appropriate to achieve bonding to the concrete.
- B. Steel work exposed to weather when construction is completed shall be hot-dipped galvanized (ASTM A123) unless noted otherwise. Detail and provide drainage and vent holes in all assemblies to be hot-dipped galvanized as recommended by ASTM A335.
- C. Do not paint or coat welds until quality control inspections are performed.
- D. Surface preparation shall be performed to a level of cleanliness appropriate to the specified coating and consistent with coating manufacturer's requirements.
- E. Where shop applied primer is required, the following areas shall not be coated:
 1. Steel work to be encased in concrete.
 2. Contact surfaces of steel connected with HSS.
 3. Steel to receive fireproofing (if any)
 4. Within 2 inches of field weldments.
 5. Steel items to be galvanized.

8. Erection:

- A. Erection methods, sequencing and temporary support and bracing of structural steel shall be the Contractor's responsibility.
- B. Erection shall conform to the AISC. All steel frames shall be considered non-self supporting unless noted otherwise or verified by the Contractor's engineer.
- C. Column baseplates and anchor bolts are not designed to withstand wind or other lateral loads on a partially completed structure.
- D. Anchor bolts are designed for loading after specified non-shrink grout is placed and cured. Anchor bolts are designed for shear and/or tension forces and have not been engineered for compression loads associated with the use of leveling nuts.

STRUCTURAL WOOD

1. General:

- A. All structural wood members shall be Coast Region Douglas Fir No. 2 for studs and Douglas Fir No. 1 for posts, beams and stringers or better grade (b) as noted in National Design Specifications for Stress Grade Lumber and its fastenings, unless noted otherwise.
- B. The Contractor shall furnish and install all bolts, nails, and plates shown, specified by the connector supplier, and otherwise required to complete the job.
- C. Washers shall be used under all bolt heads and nuts bearing on wood.
- D. All wood sill plates and wood in contact with concrete shall be pressure-treated.
- E. Bolts in wood shall be ASTM A307
- F. Washers shall be malleable iron washers (MIW) for wood to wood contact and 2'x 2' sq. x 3/16" plate washers at the sill plate to concrete slab connection of shear walls or braced wall panels.
- G. Nails shall be common by American or Canadian manufacturers, only.
- H. See National Design Specification for lag screws and shear plates.
- I. Light-gauge metal wood connectors shall be Simpson Strong-Tie or approved equal. Install per manufacturer's instructions unless noted otherwise. Use only the nails, bolts, or screws specified by the manufacturer to provide the maximum possible load capacity for that connector.
- J. Minimum nailing per OSBC Table 2304.3.1 nailing schedule.
- K. Plywood, oriented-strand board, wafer board. Each sheet shall bear the trademark of the American Plywood Association. All panels shall be APA performance rated panels bonded with exterior glue. Thickness and layup are as shown on the Drawings. All panels shall be Exposure 1.

2. Structural Composite Lumber (SCL)

- A. Products specified in this Section shall be manufactured in accordance with ICBO / NER approved specifications. They include:
 1. Laminated Veneer Lumber (LVL)
 2. Parallel Strand Lumber (PSL)
 3. Laminated Strand Lumber (LSL)
- B. Referenced standards are ASTM D 2559-04, 'Standard Specification for Adhesives for Structural Laminated Wood Products for Use Under (Wet Use) Exposure Conditions' and, ASTM D 5456-06, 'Standard Specification for Evaluation of Structural Composite Lumber Products'.
- C. SCL approved products shall be manufactured by:
 1. Boise Cascade Corp., Boise, Id.
 2. Louisiana Pacific Corp., Portland, Or.
 3. Red Bull LLC, Boise, Id.
- D. Selection of beam sizes shown on the Drawings were based upon the following design values:
 - a. Modulus of elasticity (E) - 2000 ksi
 - b. Bending stress (fb) - 2900 psi
 - c. Shear stress (fv) - 225 psi
- E. Provide flashing, moisture-proofing, and/or shielding as required to prevent prolonged exposure to wet conditions or temperatures

- C. Submit shop drawings based on truss configurations and on truss loads shown on Drawings and on requirements specified herein. Joint configurations may be modified to allow double cut webs. Determine member forces from exact analysis method as defined by TPI. Include the following information on submitted shop drawings:
 1. Allowable loads in lbs. per effective nail or lbs. per sq. inch for lumber and plates used as allowed by ICBO and current ICBO report number.
 2. Stress reduction factors used for plates and lumber.
 3. Top and bottom chord design loads in psf.
 4. Size, thickness, and exact location by dimension of plates.
 5. Lumber species and grades used.
 6. Combined stress ratio for each member.
 7. Seal and certification of Engineer, licensed in the State of Oregon, responsible for preparation of shop drawings.
 8. Name and trademark of Plate Manufacturer.
 9. Name and address of truss manufacturer along with Project name and address.
- D. Plates shall have a minimum bite of 2 1/2 inches on members and a minimum width of 3 inches. Size plates, nail and steel section for 125 percent for all trusses other than scissor trusses. Scissor trusses shall be designed for 150 percent of member forces. No increases in plate values shall be used for duration of loading.

4. Glued-laminated Construction:

- A. Products specified in this Section shall be manufactured in accordance with AITC approved specifications and bear their certification mark. Products furnished are noted as:
 1. Glu-lam Beams (GLB)
 2. Referenced standard is American Institute of Timber Construction / American National Standards Institute (AITC / ANSI A 1901-2002), 'Wood Products - Structural Glued Laminated Timber'.
 3. GLB wood shall be Coastal Douglas Fir and shall be manufactured by:
 1. Duco-Lam, Inc., Drain, Or.
 2. Roaboro Custom GluLam, Springfield, Or.
 3. Y 4 J Laminators, Myrtle Creek, Or.
 4. Boise Glulam LLC, Boise, Id.
 5. Western Wood Structures, Tualatin, Or.
- D. Selection of beam sizes shown on the Drawings were based upon the following design values:
 - a. Modulus of elasticity (E) - 1800 ksi
 - b. Bending stress (fb) - 2400 psi
 - c. Shear stress (fv) - 195 psi

ABBREVIATIONS

(UNLESS NOTED OTHERWISE)

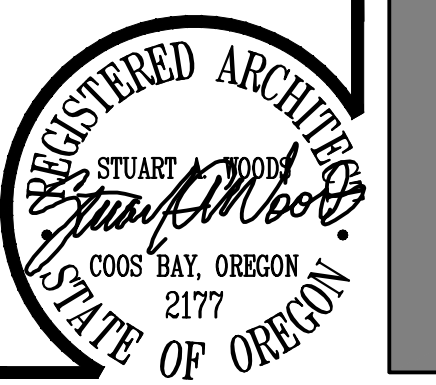
ALTERNATE	ALT	DOUBLE	DBL	LOCATION, LOCATE	LOC	ROUGH OPENING	RO
ANCHOR BOLT	AB	DRAWINGS	DRG	LONG LEG HORIZONTAL	LLH	SAUCUT CONTRACTION JOINT	SCJ
ARCHITECTURAL	ARCH	EACH	EA	MACHINE BOLT (ASPT QUALITY UNO)	MB	SCHEDULE	SCHED
BASE PLATE	BASE P	EACH FACE	EF	MATERIAL	MATL	SECTION	SECT
BEAM	BM	EACH SIDE	ES	MAXIMUM	MAX	SHEET	SHT
BEARINGS	BRG	EACH WAY	EW	MIX	MIX	SIMILAR	SIM
BLOCK	BLK	EQUAL	EQ	MIXED	MIX	SLAB-ON-GRADE	SOG
BLOCKING	BLKG	EXPANSION	EXP	METAL	METL	SPACING	SPCG
BOTH SIDES	BS	EXTERIOR	EXT	MEZZANINE	MNZ	SPECIAL CONCENTRIC BRACED FRAME	SCBF
BOTTOM	BTM	FAIR SIDE	FS	MISCELLANEOUS	MISC	SPECIFICATION	SPEC
BOTTOM OF BEAM	BOB	FINISH FLOOR	FF	NOT TO SCALE	NTS	SQUARE	SQ
BOTTOM OF FOOTING	BOF	FLANGE	FLG	OPENING	OC	STAGGERED	STAG
BRACING	BRG	FLOOR	FLR	OPPOSITE	OPP	STANDARD	STD
BUILDING	BLDG	FOOT/FEET	FT	OUTSIDE DIAMETER	OD	STEEL	STL
CAST-IN-PLACE	CIP	FOOTING	FTG	PANEL	PANL	STRUCTURAL	STRUC
CENTERLINE	CL or E	FOUNDATION	FDN	FACE OF CONCRETE	FC	TOP AND BOTTOM	T & B
CLEAR/CLEARANCE	CLR	FACE OF MASONRY	FOM	FACE OF STEEL STUD	FOS	TOP OF BEAM ELEVATION	TBE
COLUMN	COL	FACE OF CONCRETE	FC	PRECAST/PRECAST PANEL	PPC/P	TOP OF CONCRETE	TOC
CONCRETE	CONC	HEADER	HDR	PLACES	PLCS	TOP OF FOOTING	TOF
CONCRETE MASONRY UNIT	CMU	HORIZONTAL	HORIZ or H	PLATE	PLT	TOP OF MASONRY	TM
CONSTRUCTION JOINT	CJT	INCLUDE	INCL	POUNDS PER SQUARE FOOT	PPSF	TOP OF STEEL	TOST
CONTINUOUS	CONT	INSIDE DIAMETER	ID	PRESSURE-TREATED	PT	TOP OF WALL	TOW
DEAD LOAD	DL	INSIDE FACE	IF	QUANTITY	QTY	TYPICAL	TPP
DEEP	DEEP	INTERIOR	INT	REFERENCE	REF	UNLESS NOTED OTHERWISE	UNO
DIAGONAL	DIAG	INTERIOR	INT	REQUIRED	REQD	VERTICAL	VERT or V
DIAMETER	DIA or D	INTERIOR	INT	REVISION/REVISION	REV	WELDED WIRE FABRIC	WWF
DIRECTION	DIR	KIPS PER SQUARE INCH	KSI			WELDED HEADED STUDS	WHS
DITCH, DO OVER	-D/O	LIVE LOAD	LL			WELDED THREADED STUDS	WTS
						WITHOUT	W/O
						WORKING POINT	WP

SPECIAL INSPECTION PROGRAM (AS REQUIRED BY OSBC SEC. 1104)

Continuous If Marked	VERIFICATION AND INSPECTION	Periodic If Marked
[]	11042 FABRICATORS	[]
[]	1. Shop fabrication of structural load-bearing members and assemblies.	[]
[]	2. Not required if registered and approved to perform work without special inspection including:	[]
[]	11043 STEEL CONSTRUCTION	[]
[]	1. Material verification of high-strength bolts, nuts and washers	[]
[]	a. Identification markings to conform to ASTM standards specified in the approved construction documents.	[]
[]	b. Manufacturer's certificate of compliance required.	[]
[]	2. Inspection of high-strength bolting:	[]
[]	a. Bearing-type connections.	[]
[]	b. Slip-critical (snug-tight) connections.	[]
[]	3. Material verification of structural steel:	[]
[]	a. Identification markings to conform to ASTM standards specified in the approved construction documents.	[]
[]	b. Manufacturer's certified mill test reports.	[]
[]	4. Material verification of used filler materials:	[]
[]	a. Identification markings to conform to ASTM standards specified in the approved construction documents.	[]
[]	b. Manufacturer's certificate of compliance required.	[]
[]	5. Inspection of welding (in the field):	[]
[]	a. Structural steel:	[]
[]	1) Complete and partial penetration groove welds.	[]
[]	2) Multi-pass fillet welds.	[]
[]	3) Single-pass fillet welds less than 5/16".	[]
[]	4) Single-pass fillet welds greater than 5/16".	[]
[]	5) Floor and roof deck welds.	[]
[]	b. Reinforcing steel:	[]
[]	1) Verification of weldability of reinforcing steel other than ASTM A 706.	[]
[]	2) Reinforcing steel-welding flexural and axial forces in immediate and special moment frames, and boundary elements of special reinforced concrete shear walls and shear reinforcement.	[]
[]	3) Shear reinforcement.	[]
[]	4) Other reinforcing steel.	[]
[]	6. Inspection of steel frame joint details for compliance with approved construction documents:	[]
[]	a. Details such as bracing and stiffening.	[]
[]	b. Member locations.	[]
[]	c. Application of joint details at each connection.	[]
[]	11044 CONCRETE	[]
[]	1. Inspection of reinforcing steel, including prestressing tendons and placement.	[]
[]	2. Inspection of reinforcing steel welding in accordance with Table 11043, Item 5c.	[]
[]	3. Inspect bolts to be installed in concrete prior to and during placement of concrete where allowable loads are increased.	[]
[]	4. Verifying use of required design mix.	[]
[]	5. Sample fresh concrete (5 cu./yd. min.) to fabricate specimens for strength tests, perform slump and air content tests, and determine the temperature of the concrete.	[]
[]	6. Inspection of concrete and shotcrete placement for proper application techniques.	[]
[]	7. Inspection for maintenance of specified curing temperature and technique.	[]
[]	8. Inspection of prestressed concrete:	[]
[]	a. Application of prestressing forces.	[]
[]	b. Grouting of bonded prestressing tendons in seismic-force-resisting system.	[]
[]	9. Erection of precast concrete members.	[]
[]	10. Verification of in-situ concrete strength prior to stressing of tendons in post-tensioned concrete and prior to removal of shoring and forms from beam and structural slabs.	[]
[]	11. Inspect formwork for shape, location and dimensions of the concrete member being formed.	[]
[]	11045 MASONRY (LEVEL 1 OR 2)	[]
[]	1. Compliance with required inspection provisions.	[]
[]	2. Verification of Fm and Fc prior to construction.	[]
[]	3. Verification of slump, flow and VSI as delivered to the site.	[]
[]	4. Slab, mortar mix, joints, and reinforcement compliance.	[]
[]	5. Slab: Size and location of anchor bolts and reinforcing bars.	[]
[]	6. Slab: Grout mix, prep of grout cells and reinf. compliance.	[]
[]	7. Grout placement field verification.	[]
[]	8. Preparation of grout and mortar specimens and/or priens.	[]
[]	11046 WOOD CONSTRUCTION	[]
[]	1. Inspect the fabrication process of prefabricated wood structural elements and assemblies per Section 11042.	[]
[]	2. Inspect the wood structural panel sheathing, framing members, and nailing of high-strength cleopans as specified and described with Table 11043.3 of OSBC.	[]
[]	11047 BOLLS	[]
[]	1. Verify materials below footing are adequate to achieve the design bearing capacity.	[]
[]	2. Verify excavations are extended to proper depth and have reached proper material.	[]
[]	3. Perform classification and testing of controlled fill materials.	[]
[]	4. Verify use of proper materials, densities and lift thicknesses during placement and compaction of controlled fill.	[]
[]	5. Prior to placement to controlled fill, observe subgrade and verify that site has been properly prepared.	[]
[]	11048 PILE FOUNDATIONS	NA
[]	11049 PIER FOUNDATIONS	[]
[]	1. Observe drilling operations and maintain complete and accurate records for each pier.	[]
[]	2. Verify placement locations and plumbness, confirm pier diameters, ball diameters lengths, embedment into bedrock and adequate and bearing strata capacity.	[]
[]	3. For concrete piers, perform additional inspections in accordance with Section 11044.	[]
[]	4. For masonry piers, perform additional inspections in accordance with Section 11045.	[]
[]	11040 FLAME-RESISTANT MATERIALS	NA
[]	11041 MASTIC AND INTERPENETRANT PAR COATINGS	NA
[]	11042 EXTERIOR INSULATION AND FINISH SYSTEM	NA
[]	11043 SPECIAL CASES	NA
[]	11044 SPOKE CONTROL	NA

PHASE SEVEN ADDITION

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DATES:
FEB. 20, 2023
PLAN REVIEW 6/25/23
ISSUE/BID 5/10/23
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REVISION A B/30/23
REVISION
REVISION

PROJECT #
1529.7

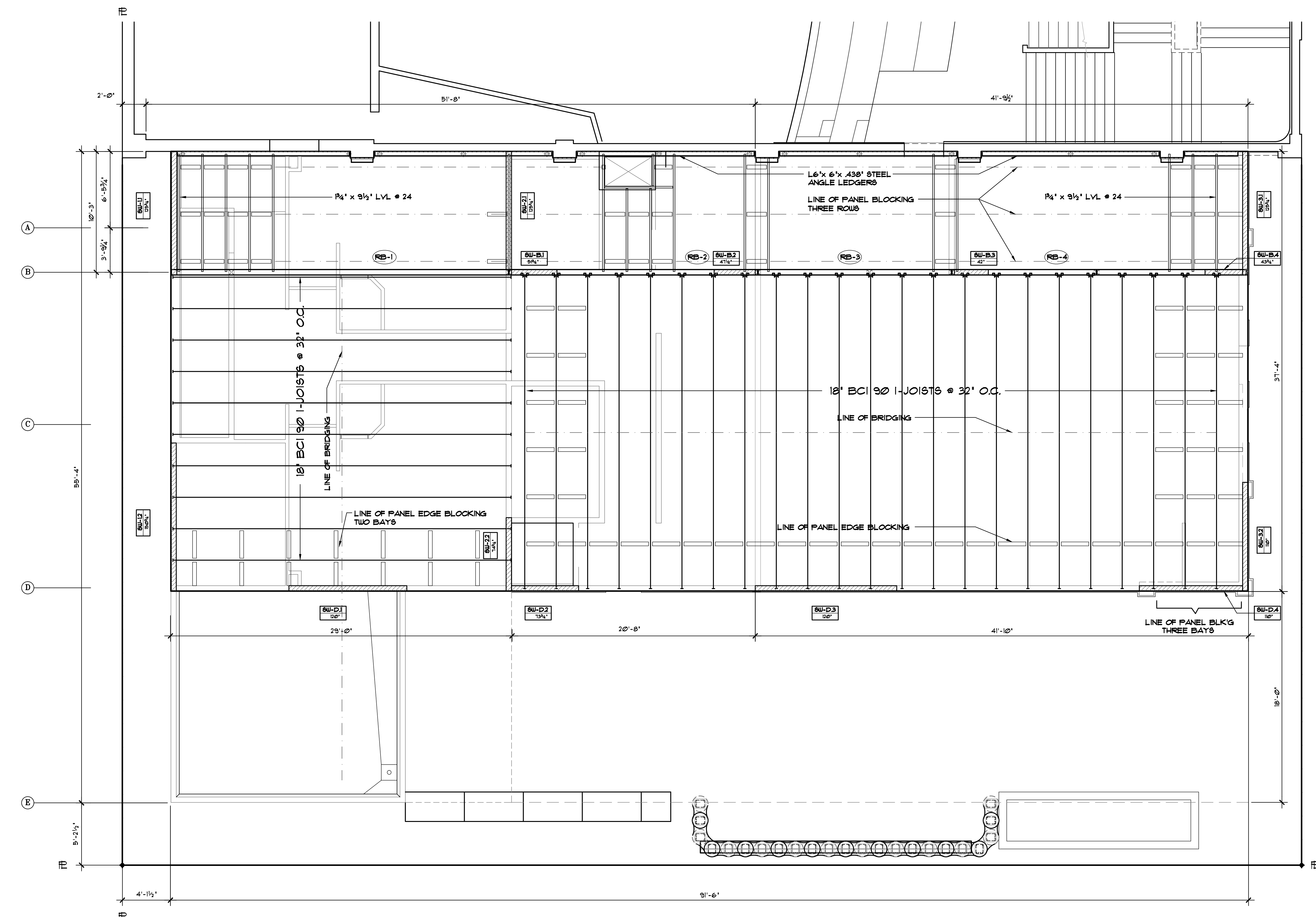
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MCSWAIN - WOODS ARCHITECTURE

PROJECT TITLE:
YOUTH THEATRE / PROGRAM CENTER
LIBRARY THEATRE - PHASE FOUR
2100 SHERMAN AVE.
NORTH BEND, OREGON 97459

432 N. 4TH STREET, COOS BAY, OREGON
(503) 705-3440 Email: stuartwoods@woods.com

STRUCTURAL GENERAL NOTES
ABBREVIATIONS
SPECIAL INSPECTION PROGRAM

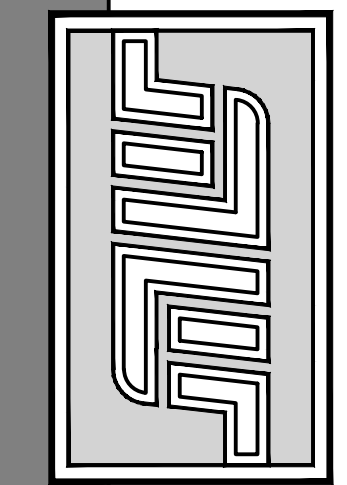


ROOF FRAMING PLAN

SCALE: 1/4" = 1'-0"

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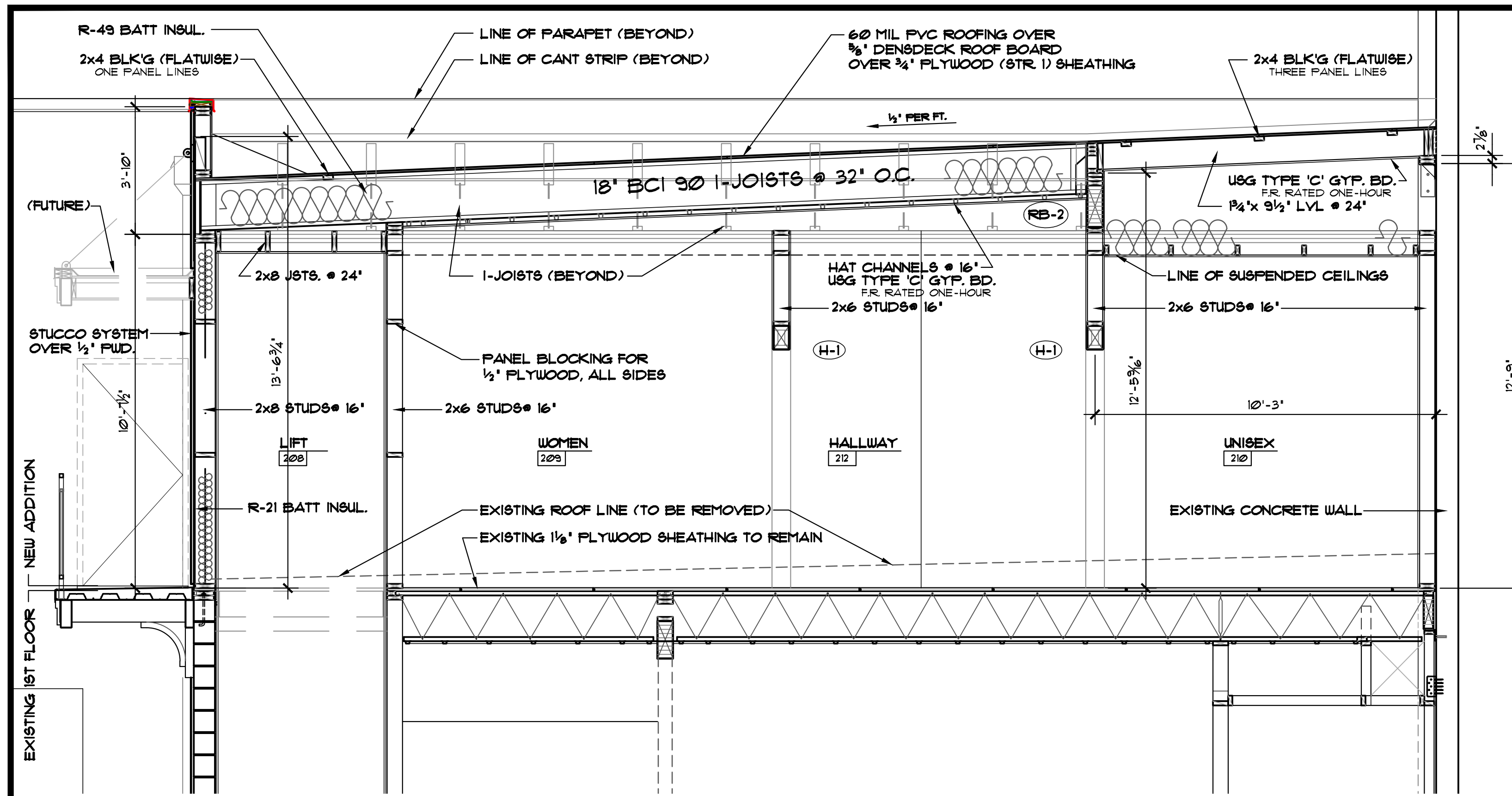
McSWAIN - WOODS ARCHITECTURE
432 N. 4TH STREET, COOS BAY, OREGON
(503) 541.269.0618 (M) 503.708.3440 Email architects@mcswain-woods.com

SHEET TITLE:
ROOF FRAMING PLAN

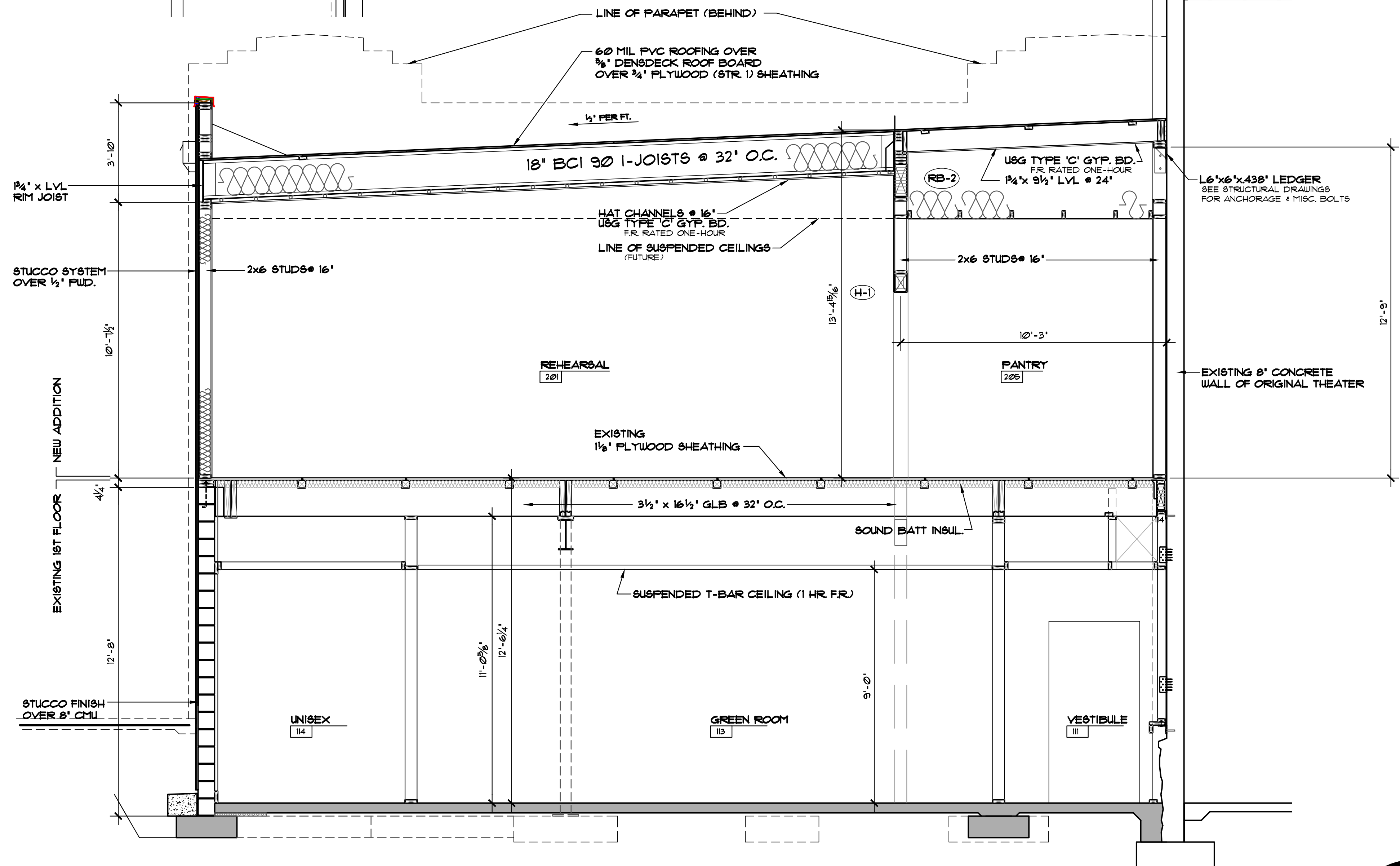
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PLAN REVIEW:	6/25/23
ISSUE/BID:	5/10/23
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REVISION:	8/30/23
REVISION:	

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S1.1



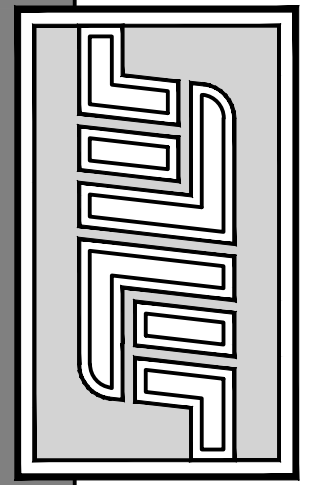
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1 BUILDING SECTION - TRANSVERSE
 SCALE: 3/8" = 1'-0"

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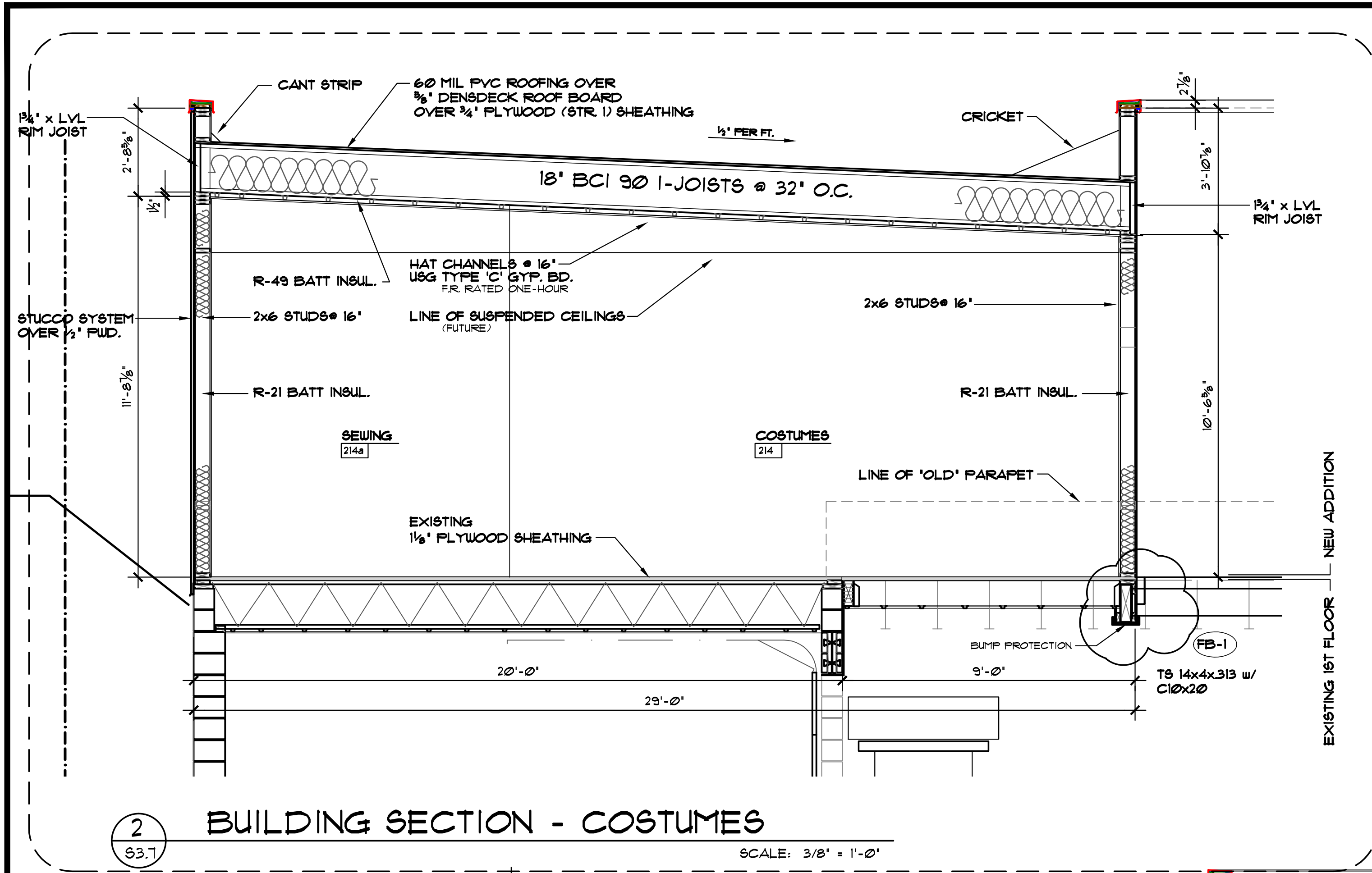
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 (O) 541.269.0618 (M) 503.708.3440 Email: stu@mcswain-woods.com

SHEET TITLE:
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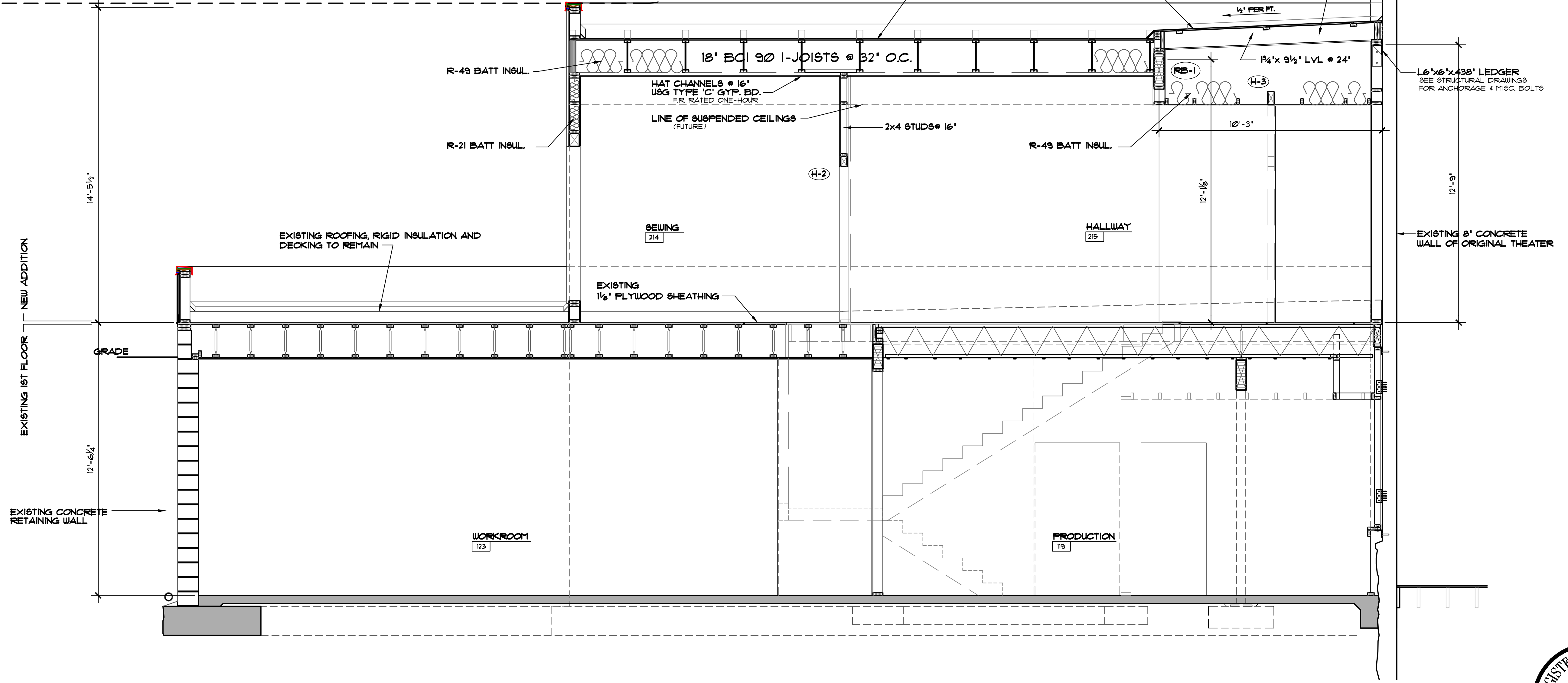
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PLAN REVIEW:	6/25/23
ISSUE/BID:	5/10/23
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S3.6



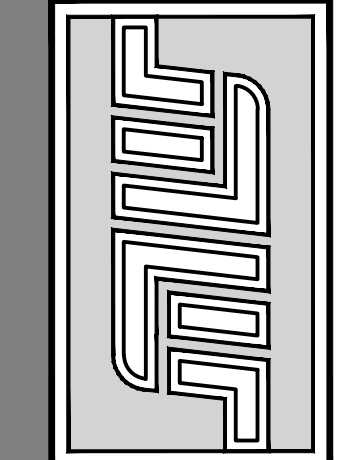
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SCALE: 3/8" = 1'-0"



1 BUILDING SECTION - TRANSVERSE
SCALE: 3/8" = 1'-0"

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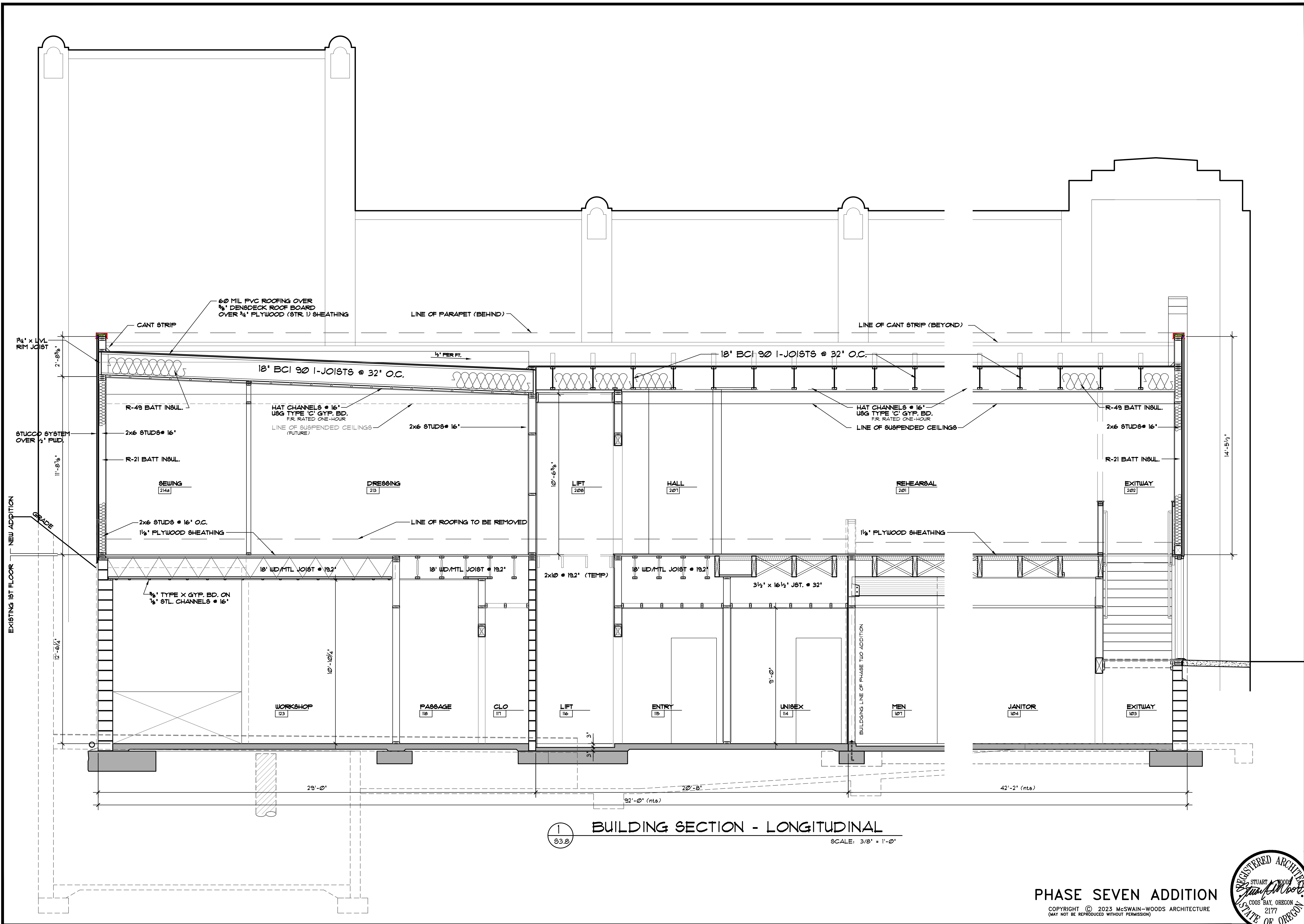
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TRANSVERSE

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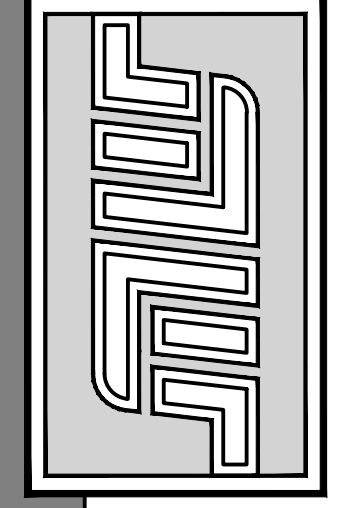
S3.7



1
S3.8

BUILDING SECTION - LONGITUDINAL

SCALE: 3/8" = 1'-0"



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SHEET TITLE:
 BUILDING SECTIONS
 LONGITUDINAL

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REVISION C	

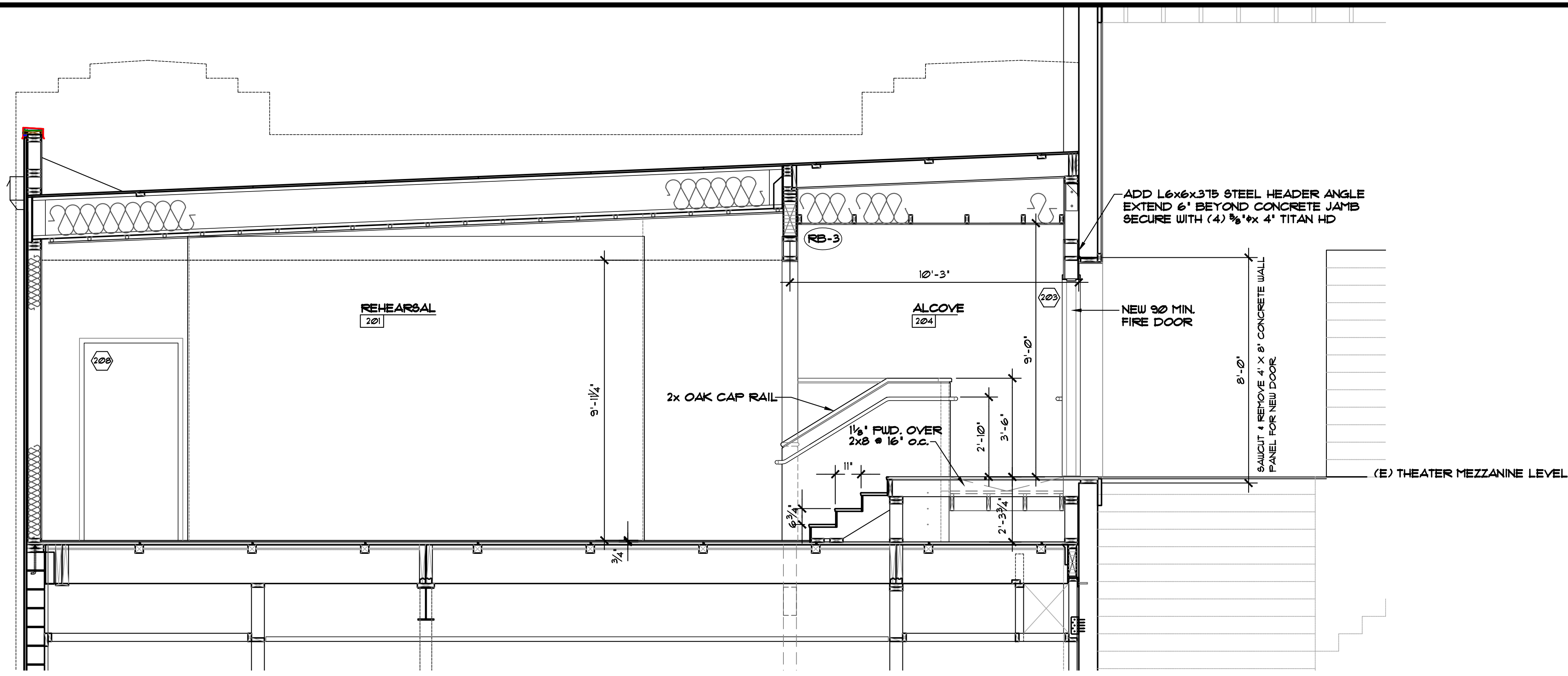
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S3.8

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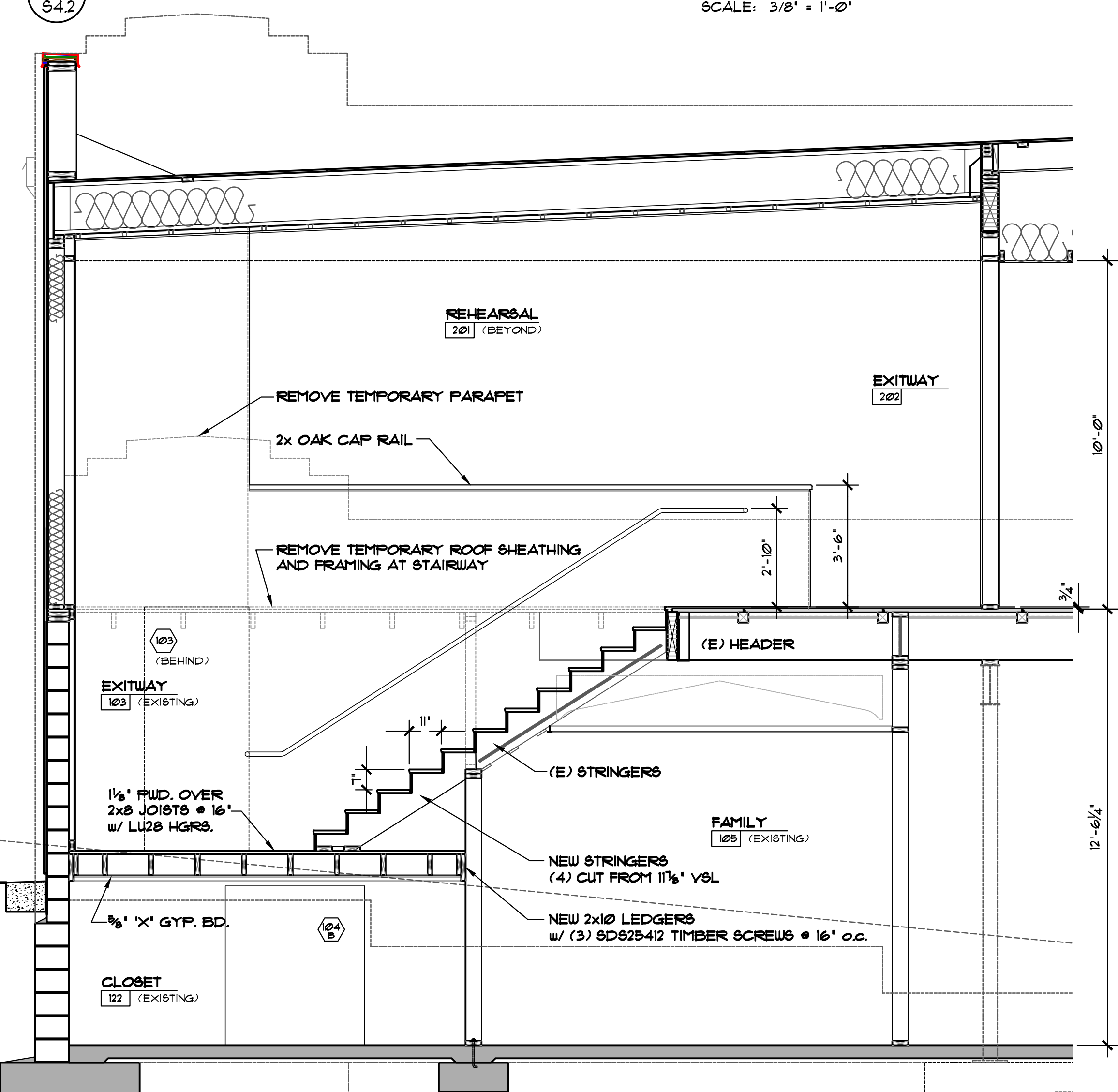
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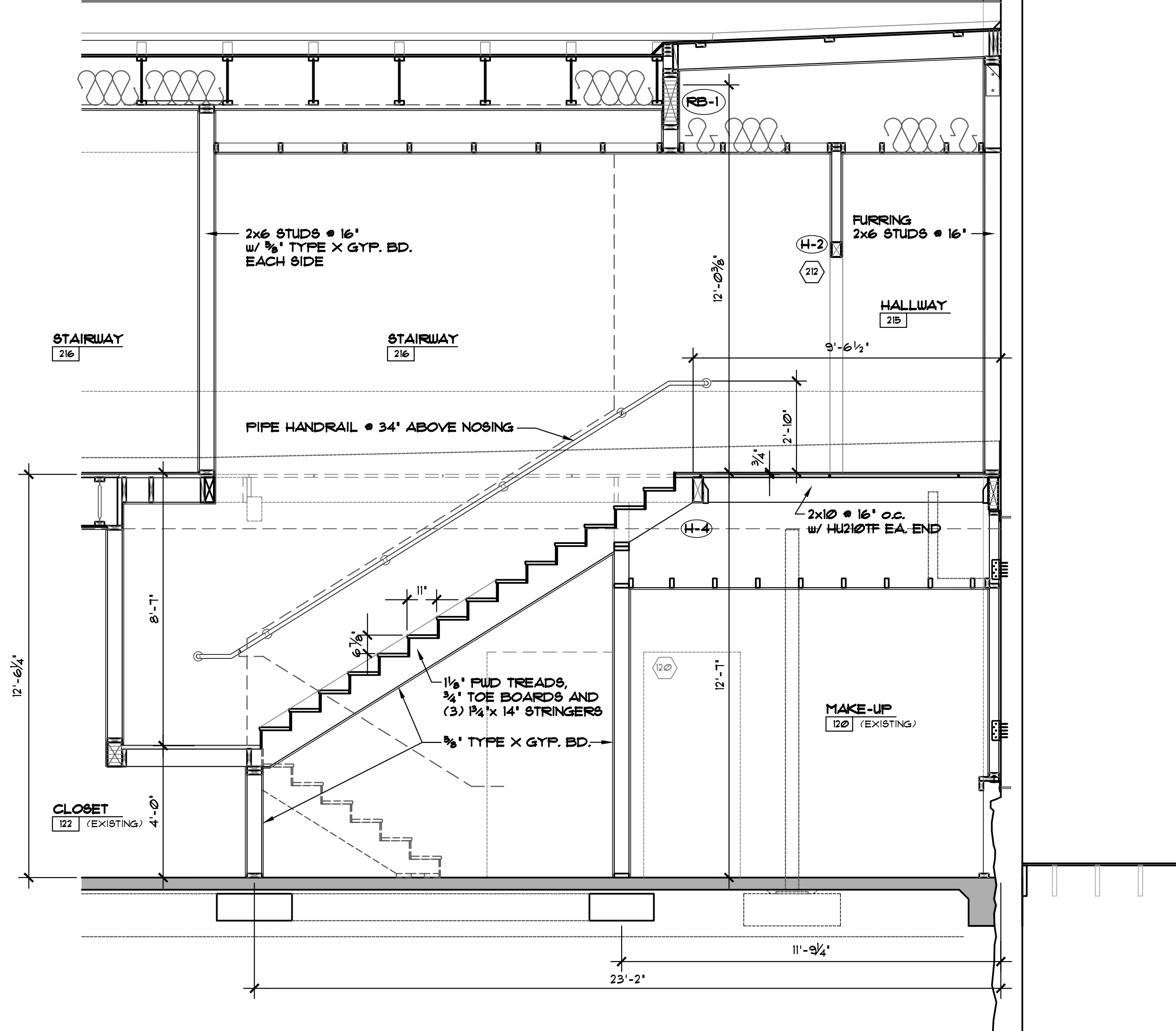
2 ALCOVE SECTION - RM. 204

SCALE: 3/8" = 1'-0"



3 EXITWAY SECTION - RM. 202

SCALE: 3/8" = 1'-0"



1 STAIRWAY SECTION - RM. 216

SCALE: 3/8" = 1'-0"

BEAM / HEADER SCHEDULE						
MARK	TYPE	SIZE	BEARING R#	BEARING R#	BEARING R#	EL. TOP
H-1	WD	(2) 2x6 BOX	JACK STUD B $\frac{1}{2}$ x I $\frac{1}{2}$	JACK STUD B $\frac{1}{2}$ x I $\frac{1}{2}$		18.54'
H-2	WD	4 x 6	JACK STUD B $\frac{1}{2}$ x I $\frac{1}{2}$	JACK STUD B $\frac{1}{2}$ x I $\frac{1}{2}$		18.29'
H-3	WD	4 x 8	JACK STUD B $\frac{1}{2}$ x I $\frac{1}{2}$	JACK STUD B $\frac{1}{2}$ x I $\frac{1}{2}$		18.44'
H-4	WD	4 x 10	BEAM HGR. H410TF	POST CAP BC4		18.60'
RB-1	GLB	5 1/2 x 18	BEAM HGR. GL18	POST CAP CC66	COLUPN CAP CC66	18.95'
RB-2	GLB	5 1/2 x 18	COLUPN CAP CC66	COLUPN CAP CC66		18.95'
RB-3	GLB	5 1/2 x 15	COLUPN CAP CC66	COLUPN CAP CC66		18.70'
RB-4	GLB	5 1/2 x 15	COLUPN CAP CC66	POCKET B $\frac{1}{2}$ x 3		18.70'
FB-1	OTL	H88 14x4x3/32	OTL. PLATE 8x4x3/32 TOP FLG	OTL. PLATE 8x4x3/32 TOP FLG		6.138'

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SHEET TITLE:
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S4.2

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